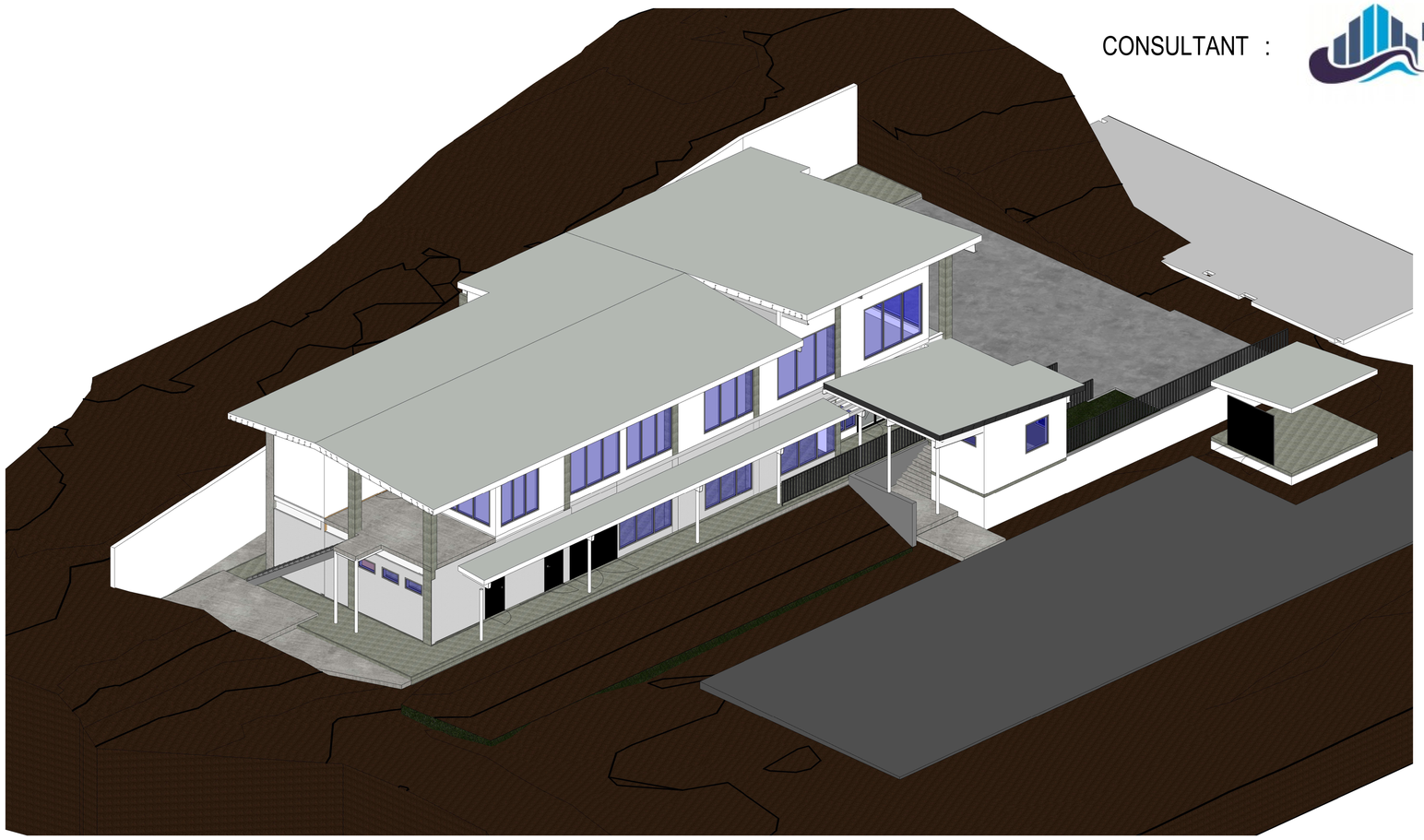
STRUCTURAL DRAWING SHEET				
SHEET NUMBER	SHEET NAME	CURRENT REVISION	SHEET ISSUE DATE	SHEET COLLECTION
G00	COVER SHEET	A	08/14/25	STRUCTURAL
G01	GENERAL NOTES (SHEET 1 OF 4)	A	08/14/25	STRUCTURAL
G02	GENERAL NOTES (SHEET 2 OF 4)	A	08/14/25	STRUCTURAL
G03	GENERAL NOTES (SHEET 3 OF 4)	A	08/14/25	STRUCTURAL
G04	GENERAL NOTES (SHEET 4 OF 4)	A	08/14/25	STRUCTURAL
S01	FOUNDATION LAYOUT PLAN	A	08/14/25	STRUCTURAL
S02	GROUND FLOOR LAYOUT PLAN	A	08/14/25	STRUCTURAL
S03	FIRST FLOOR SLAB LAYOUT PLAN	A	08/14/25	STRUCTURAL
S04	FIRST FLOOR FRAMING LAYOUT PLAN	A	08/14/25	STRUCTURAL
S05	LOWER ROOF FRAMING LAYOUT PLAN	A	08/14/25	STRUCTURAL
S06	UPPER ROOF FRAMING LAYOUT PLAN	A	08/14/25	STRUCTURAL
S20	ELEVATIONS (SHEET 1 OF 2)	A	08/14/25	STRUCTURAL
S21	SECTION (SHEET 1 OF 3)	A	08/14/25	STRUCTURAL
S23	SECTION (SHEET 2 OF 3)	A	08/14/25	STRUCTURAL
S24	SECTION (SHEET 3 OF 3)	A	08/14/25	STRUCTURAL
S30	TYPICAL P1/C1 FOOTING DETAILS	A	08/14/25	STRUCTURAL
S31	TYPICAL P2/C1 FOOTING DETAILS	A	08/14/25	STRUCTURAL
S32	TYPICAL P3/C1 FOOTING DETAILS	A	08/14/25	STRUCTURAL
S33	TYPICAL P4/C1 FOOTING DETAILS	A	08/14/25	STRUCTURAL
S34	TYPICAL P4/C2 FOOTING DETAILS	A	08/14/25	STRUCTURAL
S35	TYPICAL P5/C2 FOOTING DETAILS	A	08/14/25	STRUCTURAL
S36	TYPICAL ENCASEMENT DETAIL	A	08/14/25	STRUCTURAL
S40	TYPICAL LEVEL 1 FLOOR BEAM DETAILS	A	08/14/25	STRUCTURAL
S41	TYPICAL B1/B2 TO C1 DETAILS	A	08/14/25	STRUCTURAL
S42	TYPICAL B1/B2 TO C2 DETAILS	A	08/14/25	STRUCTURAL
S43	TYPICAL FLOOR FRAMING DETAILS	A	08/14/25	STRUCTURAL
S50	TYPICAL ROOF DETAILS (SHEET 1 OF 2)	A	08/14/25	STRUCTURAL
S51	TYPICAL ROOF DETAILS (SHEET 2 OF 2)	A	08/14/25	STRUCTURAL
S60	TYPICAL DETAILS	A	08/14/25	STRUCTURAL
S70	TYPICAL R.C STAIRCASE DETAILS	A	08/14/25	STRUCTURAL
S71	TYPICAL EMERGENCY STAIRCASE DETAILS	A	08/14/25	STRUCTURAL

PROJECT : 22507209_CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

JOB NO : 22507209

CLIENT : fmc PACIFIC

CONSULTANT : NRW MACALLAN (FIJI) LTD CONSULTING ENGINEERS *Tailor making your solutions*



FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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Client: fmc PACIFIC

NRW MACALLAN (FIJI) LTD CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE	
Checked: RC	Date: 08/14/25	CAD File:	
Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	
		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name: CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

Drawing Title: COVER SHEET

Job No: 22507209

Drawing No: G00

Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A

GENERAL NOTES - 1

GENERAL

G1. THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS' DRAWINGS, AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCY SHALL BE REFERRED TO THE SUPERINTENDENTS FOR DECISION BEFORE PROCEEDING WITH THE WORK.

G2. ALL DIMENSIONS ARE IN MILLIMETERS. DIMENSION SHALL **NOT** BE OBTAINED BY SCALING THE STRUCTURAL DRAWINGS, LEVEL SHOWN ON THE STRUCTURAL DRAWINGS ARE THE TOP OF STRUCTURAL CONCRETE OR STRUCTURAL STEELWORKS UNLESS NOTED OTHERWISE.

G3. SETTING OUT DIMENSION SHOWN ON THE DRAWINGS SHALL BE VERIFIED BY THE BUILDER ON SITE BEFORE CONSTRUCTION & FABRICATION. THE ENGINEERS DRAWINGS **ARE NOT TO BE SCALED**, NO RESPONSIBILITY WILL BE TAKEN BY THE ENGINEER FOR DIMENSIONS OBTAINED BY SCALING THE DRAWINGS

G4. DURING CONSTRUCTION THE STRUCTURE SHALL BE MAINTAINED IN A STABLE CONDITION AND NO PART SHALL BE OVERSTRESSED. TEMPORARY BRACING SHALL BE PROVIDED BY THE BUILDER AS REQUIRED.

G5. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT REQUIREMENTS OF THE AS AND NZS CODES AND THE BY-LAWS AND ORDINANCES OF THE RELEVANT BUILDING AUTHORITIES. EXCEPT WHERE VARIED BY THE PROJECT SPECIFICATION.

G6. REFER ARCHITECTURAL DRAWING FOR THE BLOCKWALL THICKNESS WHERE NOT MENTIONED ON THESE DRAWINGS AND FOR FALLS IN SLAB, EXTRA PACKING, WATERPROOFING MEMBRANES, CONTRACTION JOINT FILLING MATERIALS AND ALL OTHER ARCHITECTURAL FEATURES SUCH DRIP GROOVES, POUR BREAKS IN OFF-FORM CONCRETE, FILLETS AND THE LIKE.

G7. THE STRUCTURAL WORKS SHOWN ON THE THESE DRAWINGS HAD BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS IN ACCORDANCE WITH **AS/NZS 1170.1.2002**

FLOOR USAGE	LIVE LOAD (kPa)
GROUND FLOOR MEP & PLANT ROOM	5.0 kPa
SUSPENDED FLOOR	3.0 kPa (Office)
STAIRS & LANDINGS	4.0 kPa

NOTE:
A SUPERIMPOSED DEAD LOAD OF 1.0 kPa HAS BEEN ALLOWED FOR PARTITION & SERVICES. THE SELF-WEIGHT OF THE STRUCTURE IS NOT INCLUDED IN THESE VALUES & MUST BE CALCULATED SEPARATELY.

G8. THE STRUCTURAL WORK SHOWN ON THESE DRAWINGS HAS BEEN DESIGNED FOR THE FOLLOWING WIND LOAD IN ACCORDANCE WITH **AS 1170 PART 2 - 2021**
BASIC WIND VELOCITY: 69 M/S ULS AT 50 YEAR DESIGN LIFE
47M/S SLS AT 25 YEAR DESIGN LIFE
TERRAIN CATEGORY: 2.0
PROBABILITY OF EXCEDDANCE: 1/500
REGION: C Cpi: INTERNAL PRESSURE Cpi= 0.0, -0.30

G9. THE STRUCTURAL WORK SHOWN ON THESE DRAWINGS HAS BEEN DESIGNED FOR EARTHQUAKE LOADS IN ACCORDANCE WITH **NZS 1170.5:2004**
IMPORTANCE LEVEL = 2.0
PROBABILITY OF EXCEEDANCE = 1/500
ZONE HAZARD FACTOR, Z = 1.0 (Refer to Geotechnical Report - NAL Civil)
SITE SUBSOIL CLASS = A
STRUCTURE TYPE = NORMINAL DUCTILE

CIVIL NOTES
BEARING CAPACITY OF THE SOIL, THE FOLLOWING VALUES HAVE BEEN ASSUMED FOR SOIL STRENGTH AT SITE:
- MINIMUM CBR VALUE OF 8%
- ALLOWABLE BEARING CAPACITY OF 120 KPa
- SITE SUBSOIL CLASS A

G10. CONTRACTOR TO PRICE FOR HIGHEST AMBIGUITY IN CASE OF ANY DISCREPANCY IN DRAWINGS.

G11. THE MINIMUM LATERAL RESISTANCE LOAD OF 1.0% OF (G+UcQ) FOR EACH LEVEL, SHALL BE APPLIED SIMULTANEOUSLY AT EACH LEVEL FOR A GIVEN DIRECTION.

G12. SUBSTITUTIONS MUST BE APPROVED BY THE ENGINEER AND BE INCLUDED IN ANY TENDER SUBMISSION. ALL SUBSTITUTIONS SHALL ACHIEVE THE REQUIRED DESIGN LIFE FOR THE WORKS.

G13. DURING CONSTRUCTION THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE STRUCTURE IN A STABLE CONDITION; ENSURING NO PART SHALL BE OVER-STRESSED DURING CONSTRUCTION ACTIVITIES.

G14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND ERECTION OF ALL TEMPORARY WORKS.

G15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DELIVERY, FABRICATION, CONSTRUCTION AND ERECTION OF ALL ELEMENTS OF THE STRUCTURE.

GENERALCONT.

G16. THE STRUCTURAL DRAWINGS DO NOT SHOW ALL DETAILS OF FIXTURES, INSERTS, SLEEVES, OPENINGS, ETC. REQUIRED BY THE VARIOUS TRADES. ALL SUCH DETAILS, INCLUDING OPENINGS FOR CONSTRUCTION PURPOSES, MUST BE APPROVED BY THE ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION. THE CONTRACTOR SHALL ALLOW FOR ADDITIONAL SUPPORT MEMBERS WHERE REQUIRED.

G17. ANY DAMAGED STRUCTURAL MATERIAL SHALL BE REPAIRED OR REPLACED TO THE ENGINEERS SATISFACTION ENSURING THE ORIGINAL DURABILITY STRENGTH AND SERVICEABILITY CHARACTERISTICS ARE MAINTAINED.

G18. SHOULD ADDITIONAL SERVICES OR CHANGES TO THE STRUCTURAL DOCUMENTATION BE REQUESTED BY THE CONTRACTOR, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ASSOCIATED CONSULTANT FEES AND EXPENSES.

G19. THE CONTRACTOR SHALL ALLOW FOR THE PREPARATION AND COST OF DETAILED SHOP DRAWINGS. THE CONTRACTOR SHALL CHECK AND COORDINATE THE SHOP DRAWINGS WITH ALL TRADES, OTHER CONSULTANT DOCUMENTS AND SITE CONDITIONS.

G20. THE STRUCTURAL ENGINEER DOES NOT PROVIDE "AS BUILT" DRAWINGS FOR THE STRUCTURE THAT REFLECT THE AS BUILT CONDITIONS AT THE COMPLETION OF CONSTRUCTION. SHOULD THE CONTRACTOR REQUEST ASSISTANCE FROM THE STRUCTURAL ENGINEER FOR PRODUCTION OF "AS BUILT" DRAWINGS THEY SHALL BE RESPONSIBLE FOR ALL ASSOCIATED CONSULTANT FEES AND EXPENSES.

G21. THE CONTRACTOR SHALL ENGAGE A GEOTECHNICAL ENGINEER FOR THE PURPOSE OF CERTIFYING THE REQUIRED GROUND CONDITIONS AS NOMINATED IN THE DOCUMENTS.

G22. THE CONTRACTOR SHALL ENGAGE AN INDEPENDENT WELDING INSPECTION AND SITE SUPERVISION SERVICE TO CARRY OUT ALL STEELWORK TESTING AND EXAMINATIONS REQUIRED BY NEW ZEALAND STANDARDS. MINIMUM STEEL EXAMINATIONS REQUIRED BY THIS CONTRACT ARE :
- 100% VISUAL WELD INSPECTIONS
- 10% RADIOGRAPHIC OR ULTRASONIC WELD INSPECTIONS
- 100% BOLT TIGHTENING INSPECTION AFTER ERECTION

G23. PRIOR TO COMMENCING SITE WORKS, THE CONTRACTOR SHALL CONFIRM AND LOCATE ALL EXISTING STRUCTURES, SERVICES AND OTHER ASSETS THAT MAY BE AFFECTED BY THE WORKS. THE CONTRACTOR SHALL SEEK DIRECTION FROM THE ENGINEER SHOULD ANY EXISTING STRUCTURES, SERVICES OR ASSETS BE DIFFERENT TO THOSE SHOWN ON THE CONSULTANTS DOCUMENTS.

G24. THE DESIGN LIFE OF THE WORKS SHALL BE 50 YEARS OR AS OTHERWISE AGREED WITH THE CLIENT. ALL MATERIALS AND WORKMANSHIP PROVIDED BY THE CONTRACTOR SHALL ENSURE THAT THE REQUIRED DESIGN LIFE IS ACHIEVED.

SITE SUPERVISION AND INSPECTION NOTES

SI1. THE CONTRACTOR SHALL ENSURE A SUITABLY QUALIFIED, REGISTERED BUILDING PRACTITIONER CARRIES OUT ALL THE REQUIRED INSPECTIONS, AS SPECIFIED BY THE BUILDING SURVEYOR, TO ENABLE IT TO PROVIDE A CERTIFICATE OF COMPLIANCE - CONSTRUCTION AT THE CONCLUSION OF THE WORKS.

SI2. NRW MACALLAN HAS ALLOWED FOR STRUCTURAL INSPECTIONS TO SATISFY OUR PROFESSIONAL OBLIGATIONS IN ASSISTING THE CONTRACTOR TO FOLLOW THE DESIGN INTENT OF OUR DOCUMENTS BUT NOT NECESSARILY TO INSPECT ALL ELEMENTS.

SI3. THE CONTRACTOR SHALL GIVE A MINIMUM OF TWO (2) WORKING DAYS NOTICE PRIOR TO INSPECTIONS BEING REQUIRED. SCHEDULED INSPECTIONS SHOULD BE CANCELED A MINIMUM OF FOUR (4) WORKING HOURS PRIOR TO THE SCHEDULED TIME IF WORK WILL NOT BE READY FOR INSPECTION. IF AN AREA OF WORK IS NOT COMPLETED AT THE SCHEDULED TIME OF INSPECTION, OR IT IS DEFECTIVE AND REQUIRES RE-INSPECTION, THE RE-INSPECTION SHALL BE AT THE CONTRACTORS EXPENSE. INSPECTION TIMES SHALL BE BETWEEN THE ENGINEERS WORKING HOURS OF 9AM TO 4PM MONDAY TO FRIDAY (EXCLUDING PUBLIC HOLIDAYS). GENERALLY NO INSPECTIONS SHALL BE CARRIED OUT ON SATURDAYS, SUNDAYS OR PUBLIC HOLIDAYS EXCEPT IN EMERGENCY SITUATIONS AND BY SPECIAL BOOKING ARRANGEMENTS (INCLUDING PROVISION FOR ADDITIONAL FEES FOR THE ENGINEER).

SI4. THE CONTRACTOR SHALL ALLOW IN THEIR PROGRAMME A MINIMUM PERIOD OF TEN (10) WORKING DAYS FROM RECEIPT OF THE SHOP DRAWINGS BY THE ENGINEER TO APPROVAL OF THE DRAWINGS. UNLESS OTHERWISE ARRANGED, THE DRAWINGS SHALL BE SUBMITTED PROGRESSIVELY IN PACKAGES (RELATING TO MARKING PLANS) SO AS TO ALLOW THE ENGINEER TO CHECK THE DRAWINGS IN A TIMELY MANNER.P PROVIDED BY THE CONTRACTOR SHALL ENSURE THAT THE REQUIRED DESIGN LIFE IS ACHIEVED.

CONCRETE

C1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH **NZS 3109** CURRENT EDITION WITH AMENDMENTS, EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.

C2. CONCRETE QUALITY:-

ELEMENT	SLUMP	CONCRETE TYPE	MAX. AGG. SIZE	MINIMUM CONCRETE STRENGTH F'c MPa
FOOTINGS	80	A	20	32 MPa
SLABS ON GROUND	80	A	20	32 MPa
COLUMNS	80	A	20	32 MPa
BEAMS	80	A	20	35 MPa

REFER TO SLAB NOTES FOR GENERAL SLAB THICKNESS AND COVERS. THIS SYMBOL APPLIES ELSEWHERE.

CREATION OF ANY PENETRATIONS INTO CONCRETE (INCLUDING FOR THE INSTALLATION OF FIXINGS AND ANCHORS) SHALL ONLY BE CARRIED OUT VIA THE USE OF THE EQUIPMENT, PROCEDURES AND MATERIALS THAT DO NOT DAMAGE THE EMBEDDED STEELWORK INCLUDING REINFORCEMENT AND/OR POST-TENSIONING CABLES.

REINFORCEMENT COVER AND CONCRETE STRENGTH SCHEDULE

ELEMENTS		COVER (MM) SURFACE CAST AGAINST GROUND	COVER (MM) FORMED OR FINISHED SURFACES	MINIMUN CONCRETE GRADE (MPa)
FOOTING/PILES/PILE CAPS		75	50	30/40/30
COLUMNS	INTERNAL	50	35	30
	EXTERNAL	50	35	30
WALLS	INTERNAL	50	30	30
	EXTERNAL	50	40	30
BEAMS	INTERNAL	50	30	30
	EXTERNAL	50	40	30
SLABS/ BAND BEAM	INTERNAL	30	20	30
	EXTERNAL	40	40	30

C3. SIZES OF CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES.

C4. CONCSTRUCTION JOINTS WHERE SHOWN ON THIS STRUCTURAL DRAWINGS SHALL BE WELL SCRABBLED AND PAINTED WITH EPOXY PRIOR TO POURING OF THE FRESH CONCRETE.

C5. CONSTRUCTION JOINTS WHERE NOT SHOWN BE LOCATED TO THE APPROVAL OF THE ENGINEER.

C6. BEAM DEPTHS ARE WRITTEN FIRST AND INCLUDE SLAB THICKNESS, IF ANY.

C7. PROVIDE 15 CHAMFERS TO ALL COLUMNS & BEAMS UNLESS VARIED BY ARCHITECTS DRAWINGS.

C8. PROVIDE 20mm DRIP GROOVES TO SOFFITS OF ALL EXTERNAL SLABS & BEAMS.

C9. NO PENETRATIONS, RECESSES, SLEEVES, ETC OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT THE PRIOR APPROVAL OF THE ENGINEER.

C10. PIPE OR CONDUITS SHALL NOT BE PLACED WITHIN THE CONCRETE COVER TO REINFORCEMENT WITHOUT THE APPROVAL OF THE ENGINEER. THE CONCRETE COVER TO EMBEDDED PIPES OR CONDUITS SHALL BE A MINIMUM OF 20mm.

C11. CONCRETE SIZES SHOWN DO NOT INCLUDE FINISH AND MUST NOT BE REDUCED WITHOUT THE ENGINEERS APPROVAL. DEPTHS OF BEAMS ARE GIVEN FIRST AND INCLUDE SLAB THICKNESS. SLABS AND BEAMS ARE TO BE POURED TOGETHER.

C12. CONSTRUCTION JOINTS SHALL BE PROPERLY FORMED AND USED ONLY WHERE SHOWN OR SPECIFICALLY APPROVED BY THE ENGINEER.

C13. ALL CONCRETE SHALL BE GRADE N32, NORMAL DENSITY UNLESS NOTED OTHERWISE. MAXIMUM AGGREGATE SIZE 20mm. ADMIXTURES SHALL NOT BE USED WITHOUT THE PRIOR APPROVAL OF THE ENGINEER. MAXIMUM BASIC SHRINKAGE AT 56 DAYS SHALL BE 650 MICROSTRAIN.

C14. ALL CEMENT IS TO BE "GP" GENERAL PURPOSE PORTLAND CEMENT OR "GB" GENERAL PURPOSE BLENDED CEMENT OR TYPE "SR" SULPHATE-RESISTING CEMENT AS REQUIRED OTHERWISE ON THE DRAWINGS. EXTRA RAPID HARDENING SUPERSULPHATED AND HIGH ALUMINA CEMENTS SHALL NOT BE USED. THE USE OF FLY ASH AND/OR SILICA FUME AS A CEMENT SUBSTITUTE, OTHER THAN THAT PROPORTION ALLOWED AS PART OF THE "GB" CEMENT CONTENT WILL ONLY BE PERMITTED AS PART OF A DESIGNED CONCRETE MIX WHICH HAS BEEN APPROVED IN WRITING BY THE ENGINEER.

CONCRETE CONT.

C15. EXTERNAL CONCRETE ELEMENTS SHALL BE MINIMUM GRADE S32 MEETING THE FOLLOWING REQUIREMENTS:
(a) MINIMUM PORTLAND CEMENT CONTENT 400kg/m (NO FLY ASH TO BE USED)
(b) MAXIMUM WATER/CEMENT RATIO 0.5
(c) SHRINKAGE LIMIT 650 MICROSTRAIN AFTER 56 DAYS
(d) CHLORIDE CONTENT TO BE RESTRICTED AS PER NZS3109 (0.2% BY WEIGHT OF CEMENT)
(e) NO SALT SHALL BE ADDED
(f) SULPHATE CONTENT 5% BY WEIGHT OF CEMENT
(g) FLEXURAL TENSILE STRENGTH 3.4MPa EXTERNAL ELEMENTS ARE THOSE EXPOSED TO WEATHER OR WATER PENETRATION, NZS3101 EXPOSURE CLASSIFICATION B2 UNLESS NOTED OTHERWISE.

C16. FREE DROPPING OF CONCRETE FROM A HEIGHT GREATER THAN 1000mm IS NOT PERMITTED.

C17. CAMBER TO SUSPENDED SLABS AND BEAMS SHALL BE AS NOTED ON THE DRAWINGS. NO CAMBER IS REQUIRED TO POST-TENSIONED SLABS AND BEAMS UNLESS NOTED ON THE DOCUMENTS.

C18. NO HOLES, CHASES OR EMBEDMENT OF PIPES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN THE CONCRETE MEMBERS WITHOUT THE PRIOR APPROVAL OF THE ENGINEER.

C19. ALL STEEL EMBEDMENTS SHALL BE HOT DIP GALVANIZED WITH MINIMUM ZINC COATING OF 900g/m2 (125 MICRONS).

C20. CONCRETE MUST BE CURED BY AN APPROVED METHOD IN ACCORDANCE WITH THE SPECIFICATION FOR SEVEN DAYS AFTER POURING.

C21. CONCRETE SHALL BE SEPARATED FROM SUPPORTING MASONRY BY TWO LAYERS OF MALTHOID (OR AN APPROVED EQUIVALENT). VERTICAL FACES OF CONCRETE SHALL BE KEPT FREE OF ADJOINING SURFACES BY 10mm THICKNESS OF ABLEFLEX (OR AN APPROVED EQUIVALENT). ALL NON-LOADBEARING WALLS SHALL BE KEPT CLEAR OF THE UNDERSIDE OF SLABS AND BEAMS BY 20mm UNLESS NOTED OTHERWISE.

C22. BRICKWORK MUST NOT BE BUILT ON CONCRETE SLABS OR BEAMS UNTIL FORMWORK SUPPORTING IT HAS BEEN REMOVED.

C23. HIGH FREQUENCY VIBRATORS SHALL BE USED TO COMPACT ALL CONCRETE.

C24. SURFACES RECEIVING GROUT SHALL BE LEFT ROUGH AND FREE OF LAITANCE.

C25. REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY AND NOT NECESSARILY SHOWN IN TRUE PROJECTION.

C26. COVER TO REINFORCEMENT AND CONCRETE GRADE SHALL BE AS SCHEDULED BELOW UNLESS NOTED OTHERWISE.
(a) COVER IS THE CLEAR DISTANCE BETWEEN ANY REINFORCING (INCLUDING FITMENTS) AND THE FACE OF THE STRUCTURAL ELEMENT
(b) ALL COVERS SHALL BE MAINTAINED USING APPROVED BAR CHAIRS. IN SLABS BAR CHAIRS SHALL BE AT 800x800 mm MIN. CTS. BAR CHAIRS SHALL BE PROVIDED ALONG THE EDGES OF ALL CONSTRUCTION JOINTS. STOP ENDS SHALL NOT BE USED TO MAINTAIN COVERS.
(c) FOR ALL EXTERNAL SURFACES, PROVIDE FULLY PLASTIC BAR CHAIRS. TIE WIRE SHALL NOT BE NAILED TO THE FORMS. REINFORCING BARS SHALL NOT BE USED TO KEEP FORMS APART. THROUGH TIE SYSTEM SHALL BE USED TO TIE FORMS.
(d) PROVIDE AN APPROVED VAPOUR BARRIER FOR SLABS, BEAMS AND THICKENINGS CAST AGAINST THE GROUND.

C27. REINFORCEMENT NOTATIONS;
N DENOTES DEFORMED BARS GRADE 500N TO AS/NZS 4671. NORMAL DUCTILITY (THIS MAY BE CALLED HD IN NZ).
R DENOTES PLAIN ROUND BARS GRADE 250N TO AS/NZS 4671
SL, RL DENOTES HARD-DRAWN WIRE REINFORCING FABRIC GRADE 500L TO AS/NZS 4671.
L DENOTES LOW DUCTILTIY CLASS BAR GRADE 500L TO NZS3101.
E DENOTES SEISMIC DUCTILITY CLASS BAR GRADE 500E T0 NZS3101.
HR DENOTES SESIMIC DUCTILITY CLASS GRADE 500E PLAIN BAR.
THE NUMBER IMMEDIATELY FOLLOWING THE BAR GRADE SYMBOL REPRESENTS THE NOMINAL BAR DIAMETER IN MILLIMETRES. THE FIGURE FOLLOWING THE FABRIC SYMBOL IS THE REFERENCE NUMBER.
SUBSTITUTION OF 500N REINFORCING BY 500L MESH IS NOT PERMITTED.
SUBSTITUION OF 500E REINFORCING BY 500N OR 500L REINFORCEMENT IS NOT PERMITTED.

C28. SPLICES IN REINFORCEMENT SHALL BE MADE IN THE POSITIONS SHOWN OR AS OTHERWISE APPROVED BY THE ENGINEER. GRADE 500E AND 500N BARS SHALL BE LAPPED IN ACCORDANCE WITH THE STANDARD LAP LENGTH TABLE BELOW, UNLESS NOTED OTHERWISE ON THE DRAWINGS. MINIMUM LAP FOR ALL FABRICS SHALL BE THE SPACING OF TWO TRANSVERSE WIRES PLUS 25mm.

C29. ALL REINFORCEMENT BARS TO BE MICRO-ALLOYED 500E AND 300E GRADES BARS. NO REINFORCING BARS ARE TO BE USED THAT HAVE BEEN MANUFACTURED USING THE QUENCHED AND TEMPERED PROCESS (QT).

C30. ALL REINFORCEMENT SHALL BE SUPPORTED IN ITS CORRECT POSITION ON APPROVED BAR CHAIRS AT 1000mm MAXIMUM CENTRES BOTH WAYS, SO AS NOT TO BE DISPLACED DURING CONCRETING. WHERE REQUIRED PROVIDE N16 SUPPORT BARS AT 1000mm CENTRES.

C31. UNLESS OTHERWISE SHOWN, PROVIDE N12-300 TOP & BOTTOM BOTH WAYS MINIMUM TYING STEEL WHERE REQUIRED, LAPPED 300mm AT SPLICES.

CONCRETE CONT.

C32. 2-N12 DIAGONAL CORNER BARS 1200mm LONG AS REQUIRED AT ALL RE-ENTRANT CORNER IN SLAB AND WALL (ONE FACE ONLY).

C33. REINFORCEMENT SET-OUT DIMENSIONS ARE RELATED TO COLUMN CENTERLINES, QUARTER SPAN POINTS AND BEAM EDGES U.N.O.

C34. REINFORCEMENT LENGTHS INDICATED ARE IN MILLIMETRES AND ARE PLAN LENGTH ONLY. TURN DOWNS AND CRANKS ARE NOT INCLUDED IN THE DIMENSION.

C35. BARS SHOWN STAGGERED ON PLAN SHALL BE PLACED ALTERNATELY.

C36. BARS SHALL BE EVENLY DISTRIBUTED OVER THE WIDTH OF THE STRIP U.N.O.

C37. REINFORCEMENT SHALL NOT BE CUT OR WELDED ON SITE UNLESS APPROVED BY THE ENGINEER. BARS CONFLICTING WITH SMALL HOLES AND OTHER MINOR PENETRATIONS LESS THAN 300mm LONG MAY BE DISPLACED LATERALLY.

C38. FORMWORK SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH AS 3610. NO CONCRETE SHALL BE PLACED UNTIL THE FORMWORK IS INSPECTED AND CERTIFIED BY THE FORMWORK ENGINEER AT THE CONTRACTOR'S EXPENSE.

C39. THE CLASS AND COLOUR OF THE CONCRETE SURFACE FINISH SHALL BE AS SPECIFIED ON THE ARCHITECTURAL DRAWINGS AND/OR THE SPECIFICATION. APPROVAL OF THE CONCRETE MIX DOES NOT ABSOLVE THE CONTRACTOR OF THE NEED TO COMPLY WITH THE REQUIRED CONCRETE COLOUR REQUIREMENTS.

C40. BACKPROPPING OR AN UNDISTURBED FORMWORK SHALL BE USED. RESHORING SYSTEMS ARE NOT PERMITTED.

C41. MINIMUM FORMWORK STRIPPING TIMES ARE AS A GUIDE IN AMBIENT TEMPERATURES BETWEEN 12°C AND 20°C AS FOLLOWS, PROVIDED THE TARGET CONCRETE STRENGTH IS ACHIEVED:
(a) VERTICAL SURFACES 2 DAYS (fcm = 5 MPa)
(b) SOFFITS OF BEAMS AND SLABS 6 DAYS (fcm = 22 MPa)
(c) FLOOR BACKPROPPING 18 DAYS (fcm = 28 MPa)
fcm IS THE TARGET CONCRETE STRENGTH AT THE TIME U.N.O.

C42. MINIMUM OF 3 LEVELS OF FORMWORK AND/OR BACKPROP SUPPORTS ARE REQUIRED, UNLESS NOTED OTHERWISE.

C43. HORIZONTAL LOADS ON FORMWORK ARE TO BE RESISTED BY THE FORMWORK BRACING. STRUTTING OF THE FORMWORK FROM THE PERMANENT STRUCTURE TO RESIST HORIZONTAL LOADS IS NOT PERMITTED.

C44. CREATION OF ANY PENETRATIONS INTO CONCRETE (INCLUDING FOR THE INSTALLATION OF FIXINGS AND ANCHORS) SHALL ONLY BE CARRIED OUT VIA THE USE OF EQUIPMENT, PROCEDURES AND MATERIALS THAT DO NOT DAMAGE THE EMBEDDED STEELWORK INCLUDING REINFORCEMENT AND / OR POST-TENSIONING CABLES.

C45. DPC TO BE TAPED AND PROTECTED FROM DAMAGES. DO NOT USE DAMAGED DPC. MINIMUM OF 200 MICRON DPC SHALL BE USED UNDER FOUNDATION AND SLAB ON GROUND.

C46. SCABBLE AND DAMPEN ALL CONCRETE JOINTS PRIOR TO POURING AGAINST.

C47. ALL REINFORCING IS TO BE FREE OF GREASE, OIL, MUD, etc.

C48. PLACING AND SPACING OF REINFORCEMENT IN CONCRETE - GENERAL:
(a) PLACE STARTER BARS AND REINFORCING BARS ACCURATELY.
(b) SPLICING OF REINFORCEMENT, WASHER BY LAPPING, WELDING OR MECHANICAL SPLICE, SHALL ONLY BE CARRIED OUT AS SHOWN ON THE DRAWINGS OR AS SPECIFICALLY APPROVED BY THE ENGINEER, EXCEPT AS NOTED BELOW:

- WELDED BY THE ENGINEER, EXCEPT AS NOTED BELOW:
- REINFORCEMENT IN SLABS ON GRADE AND IN TOPPINGS SHALL BE SPLICE AS REQUIRED, BUT NOT THROUGH SLAB JOINTS.

(c) LAYERS OF BEAM REINFORCEMENT SHALL BE SEPARATED WITH R32 BARS AT 1500 CRS.
(d) ALL HOOKS ON STIRRUPS & TIES MUST FIT CLOSELY AROUND MAIN BARS U.N.O. FIRST STIRRUPS TO BE PLACED NOT FURTHER THAN THE LESSER OF 1/2 STIRRUP SPACING OR 50mm FROM SUPPORT FACE.

DIMENSIONS

ALL DIMENSIONS ARE IN MILLIMETERS, EXCEPT, LEVELS AND COORDINATES WHICH ARE IN METERS. THE DECIMAL SEPARATOR USED IS THE POINT (DOT) NOT COMMA. USE GIVEN DIMENSIONS. **DO NOT SCALE OR MEASURE** OFF DRAWINGS OR CAD FILES. IF IN DOUBT, ASK.

STABILITY

MAINTAIN THE STRUCTURE IN A STABLE CONDITION DURING CONSTRUCTION. DO NOT EXCEED DESIGN LOADS SHOWN ON SPECIFIC DRAWINGS OR CAUSE ANY ELEMENT TO BE OVERSTRESSED. PROVIDE TEMPORARY BRACING AS REQUIRED.

FOR TENDER

REVISIONS

No	By	Date	Description	Ckd	Appr
A	EA	24.11.2025	FOR TENDER	RC	NK

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NRW MACALLAN (FIJI) LTD
CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE
Checked: RC	Date: 08/14/25	CAD File: / Plot By: / Plot Date:
Designed: RC	Date: ----	QUALITY ASSURANCE
Approved: NK	Date: ----	DIRECTOR / DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Drawing Title:
GENERAL NOTES (SHEET 1 OF 4)

Job No: 22507209

Drawing No: G01

Scales: A1 AS SHOWN / A3 HALF SHOWN

REVISION A

GENERAL NOTES - 2

BULK EXCAVATION NOTES:

B1. THE CONTRACTOR IS TO NOTIFY ALL SERVICE AUTHORITIES AND ARRANGE FOR DISCONNECTION OF SERVICES OR SUPPLY AS APPLICABLE INCLUDING ALL CUTTING, DISCONNECTION OR SEALING OFF OF SERVICES AND DRAINS AS REQUIRED. SERVICES OR SUPPLY LINES THAT ARE TO BE RETAINED SHALL REMAIN UNDAMAGED AND GIVEN ALL NECESSARY PROTECTION.

B2. THE BULK EXCAVATION SHALL BE CARRIED OUT TO THE BULK EXCAVATION PROFILE ON THE DRAWINGS OR TO THE UNDERSIDE OF THE STRUCTURE, INCLUSIVE OF SUB-BASE MATERIALS. THE CONTRACTOR SHALL VERIFY ANY LEVELS SHOWN ON THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL DOCUMENTS. ANY DISCREPANCY SHALL BE REFERRED TO THE ENGINEER FOR DIRECTION.

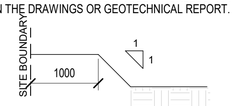
B3. UNLESS AUTHORIZED BY THE ENGINEER OVER- EXCAVATIONS SHALL NOT BE CLAIMED AS VARIATION. SUCH EXCESS EXCAVATIONS SHALL BE FILLED WITH 3% CEMENT STABILIZED SAND UNLESS COMPACTED BACKFILLING IS APPROVED BY THE ENGINEER.

B4. THE CONTRACTOR SHALL PROVIDE ALL BRACING NECESSARY TO RETAIN EARTH BANKS, ROADS, PAVEMENTS, WALLS AND FOOTINGS OF ADJOINING PROPERTIES AND PREVENT CAVING AND DISPLACEMENT OF ADJACENT SOIL OR STRUCTURES. THE CONTRACTOR SHALL PROVIDE AND DESIGN ALL NECESSARY COMPONENTS OF SUCH BRACING.

B5. ALL EXCAVATED MATERIAL SHALL BE REMOVED FROM THE SITE. THIS MATERIAL SHALL NOT BE USED FOR LEVELING, FILLING EXCESS EXCAVATIONS OR BACKFILLING BEHIND RETAINING WALLS UNLESS WRITTEN APPROVAL HAS BEEN OBTAINED FROM THE ENGINEER.

B6. THE CONTRACTOR SHALL PROVIDE ALL PUMPS, TEMPORARY SUMPS AND DRAINS AS MAY BE NECESSARY TO MAINTAIN THE EXCAVATION FREE OF STANDING WATER AT ALL TIMES.

B7. BULK EXCAVATION BATTERS AROUND THE PERIMETER OF THE SITE ARE TO BE AS SHOWN BELOW UNLESS NOTED OTHERWISE ON THE DRAWINGS OR GEOTECHNICAL REPORT.



B8. ALL GROUND UNSUITABLE FOR THE PURPOSES OF THE WORKS, SHALL BE REMOVED AND FILLED WITH 3% CEMENT STABILISED SAND OR APPROVED COMPACTED BACKFILLING AS NOMINATED BY THE GEOTECHNICAL ENGINEER

FOUNDATION NOTES:

F1. REFER TO THE GEOTECHNICAL REPORT FOR THIS PROJECT, IF AVAILABLE. WHERE NO GEOTECHNICAL REPORT EXISTS, REFER TO THE ASSUMED GEOTECHNICAL PARAMETERS NOTED IN THE DRAWINGS. ANY VARIATIONS TO THESE PARAMETERS MUST BE VERIFIED AND CONFIRMED BY A GEOTECHNICAL ENGINEER.

F2. BEFORE ANY REINFORCEMENT OR CONCRETE IS PLACED, THE SAFE BEARING CAPACITY OF THE GROUND IS TO BE VERIFIED BY THE GEOTECHNICAL ENGINEER. EXCAVATION SHALL CONTINUE UNTIL THE REQUIRED BEARING CAPACITY IS FOUND. ANY OVER-EXCAVATION SHALL BE BACKFILLED WITH 15 MPa BLINDING CONCRETE TO THE UNDERSIDE OF THE WORKS.

F3. THE DEPTHS TO UNDERSIDE OF ALL FOOTINGS ARE PROVISIONAL ONLY. AFTER EXCAVATION APPROVAL SHALL BE OBTAINED FROM THE ENGINEER FOR THE LEVELS IF NECESSARY PRIOR TO FURTHER WORK.

F4. THE FOUNDATION EXCAVATION IS TO BE KEPT FREE OF WATER AT ALL TIMES BY BAILING AND PUMPING IF NECESSARY, PARTICULARLY PRIOR TO CONCRETING. CONCRETE SHALL NOT BE PLACED IN WATER.

F5. CONCRETE BLINDING SHALL BE PLACED WITHIN 24HRS OF EXCAVATION ONCE APPROVAL HAS BEEN GIVEN BY GEOTECHNICAL ENGINEER.

F6. REFER CIVIL DRAWINGS, SPECIFICATION AND THE GEOTECHNICAL REPORT FOR SITE PREPARATION REQUIREMENTS FOR THE WORKS UNLESS NOTED OTHERWISE.

F7. THE FOUNDATION EXCAVATION IS TO BE KEPT FREE OF WATER AT ALL TIMES BY BAILING AND PUMPING IF NECESSARY, PARTICULARLY PRIOR TO CONCRETING. CONCRETE SHALL NOT BE PLACED IN WATER.

F8. ALL FOOTINGS TO BE FOUNDED A MINIMUM OF 100mm INTO THE FOUNDING MATERIAL. REFER TO THE GEOTECHNICAL REPORT FOR FURTHER DETAILS.

REINFORCEMENT

R1. REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY. IT IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.

R2. SPLICE IN THE REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN. THE WRITTEN APPROVAL OF THE ENGINEER SHALL BE OBTAINED FOR ANY OTHER SPLICES. LAP LENGTH FOR DEFORMED BARS SHALL BE AS TABULATED BELOW.

- ALL LAPS AND DEVELOPMENT LENGTHS (mm) ARE TO BE IN ACCORDANCE WITH NZS 3101:1997

CONC. 30 MPA STEEL GR. 500	BAR DIAMETER					
	HD10	HD12	HD16	HD20	HD25	HD32
HORIZONTAL WITH MORE THAN 300mm CONC. BELOW BAR	600	750	950	1200	1500	1900
FOR - COLUMNS.VERT WALL BARS.	500	600	750	950	1200	1500

CONC. 30 MPA STEEL GR. 300	BAR DIAMETER					
	D10	D12	D16	D20	D25	
HORIZONTAL WITH MORE THAN 300mm CONC. BELOW BAR	360	430	720	720	900	
FOR - COLUMNS.VERT WALL BARS.	280	350	450	550	700	

- STAGGER LAPS AS MUCH AS PRACTICABLE. TOP STEEL SHALL BE LAPPED WITHING CENTRAL HALF OF THE BEAM SPAN & BTM BEAM BARS WITHING ¼ ON EITHER SIDE OF SUPPORT UNO.
- FOR PLAIN BARS, LAP LENGTHS SHALL BE TWICE THE LENGTH AS SHOWN ABOVE, USE SEISMIC GRADE (E) REBARS.

R3. WELDING OF REINFORCEMENT WILL NOT BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS AND WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

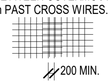
R4. ALL REINFORCEMENT FABRIC SHALL COMPLY WITH NZS 3402P AND SHALL BE SUPPLIED AS FLAT SHEETS. ALL REINFORCING MESH TO BE MICRO-ALLOYED SESIMIC GRADE 500E REINFORCING MESH U.N.O.

TYPICAL FABRIC LAP:-



WELDED WIRE MESH MADE UP OF SMOOTH WIRES SHALL BE LAP SPLICED WITH A MIN. 200 OVERLAP BETWEEN OUTERMOST CROSS WIRES THIS: LAP REINF. BARS 300mm PAST CROSS WIRES.

FOR MESH MADE UP OF DEFORMED BARS SHALL BE LAP SPLICED



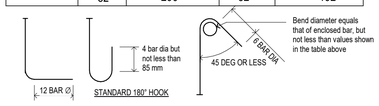
R5. PLACE SUFFICIENT BAR CHAIRS UNDER BOTTOM REINFORCING RODS AND TOP CROSSRODS IN SLABS TO ALLOW THEM TO BE SUPPORTED IN THEIR CORRECT POSITIONS DURING CONCRETING (NOT GREATER THAN 900 MM CENTRES BOTH WAYS).

R6. REINFORCEMENT LAYERS DENOTED THUS:-
TT - DENOTES TOP BARS LAID LAST
T - DENOTES TOP BARS LAID THIRD
B - DENOTES BOTTOM BARS LAID SECOND
BB - DENOTES BOTTOM BARS LAID FIRST

R7. **BENDING OF REINFORCEMENT**, BARS PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE SITE BENT, UNLESS NOTED OR SHOWN ON THE DRAWINGS OR SPECIFICALLY APPROVED BY THE ENGINEER

THE MINIMUM INTERNAL DIAMETER OF BEND OF ALL BARS SHALL BE AS FOLLOWS UNO..

STEEL GRADE	MINIMUM DIAMETER OF BEND			
	MAIN REINFORCEMENT		STIRRUPS & TIES	
	BAR Ø	MIN. Ø OF BEND	BAR Ø.	MIN.Ø. OF BEND
GRADE 300 & GRADE 500	10	50	10	40
	12	60	12	48
	16	80	16	64
	20	100	20	80
	25	150	25	150
	32	200	32	192



STANDARD HOOK STANDARD STIRRUP ANCHORAGE

- FOR PLAIN BARS, REFER NZS 3109-1997 TABLE 3-1.
- WHERE DEFORMED BARS ARE GALVANIZED BEFORE OR AFTER BENDING, THE MINIMUM BEND DIAMETER SHALL BE:

(i) 5 db FOR BAR DIAMETER OF 16mm OR LESS.
(ii) 8 dp FOR BAR DIAMETER OF 20mm OR GREATER.
ANY VARIATION TO THE ABOVE SHALL BE REFERRED TO THE ENGINEER.

STEELWORK

S1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH **AS 4100 AS 4600 & AS 1554** EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.

S2. UNLESS OTHERWISE NOTED, ALL STEEL SHALL BE IN ACCORDANCE WITH:-

AS 1204 GRADE 350 FOR ROLLED SECTIONS (UB, UC, PFC, EA, UA)
AS 1163 GRADE 350 FOR R.H.S. SECTIONS
AS 1163 GRADE 350 FOR C.H.S. SECTIONS
AS 1204 GRADE 350 FOR ALL HIGH STRENGTH STEEL.
AS 1397 GRADE 450, Z350 COLD FORM PURLINS AND GIRTS
AS 1204 GRADE Q355B FOR WELDED SECTIONS
AS 3679 PARTS 1+2 GRADE 350 FOR FLATE PRODUCTS & HOT ROLLED FLOOR PLATES. ROD BRACING TO BE GRADE 500E REINFORCING BAR U.N.O.

S3. THE BUILDER SHALL PREPARE WORKSHOP DRAWINGS AND SHALL SUBMIT THREE COPIES OF EACH DRAWINGS FOR APPROVAL. FABRICATION SHALL **NOT** COMMENCE UNTIL APPROVAL HAS BEEN RECEIVED, APPROVAL DOES NOT INCLUDE DIMENSIONS.

S4. UNLESS NOTED OTHERWISE, ALL BOLTS TO BE 20 DIAMETER COMMERCIAL GRADE STRUCTURAL BOLTS OF GRADE 8.8 SNUG TIGHT (M20-8.8/S) CONFORMING TO **AS 1111**. BOLTS – DESIGNATED BY THE NUMBER, DIAMETER, GRADE AND TIGHTENING PROCEDURE E.G.
4-M16 4.6/S MEANS 4 16 DIA. COMMERCIAL GRADE BOLTS SNUG TIGHT TO **AS1111**
TIGHTENED TO SNUG TIGHT CONDITION. 4-M16 8.8/S MEANS 4 16 DIA. HIGH STRENGTH STRUCTURAL BOLTS OF GRADE 8.8 TO **AS1252** TIGHTENED TO A SNUG TIGHT CONDITION.
6-M20 8.8 /TF MEANS 6 M20 HIGH STRENGTH STRUCTURAL BOLTS FULLY TENSIONED TO **AS4100** IN A FRICTION JOINT.
6-M24 8.8/TB MEANS 6M24 HIGH STRENGTH STRUCTURAL BOLTS FULLY TENSIONED TO **AS4100** IN A BEARING JOINT. (SOME SLIP ALLOWED) U.N.O. ALL HOLES SHALL BE DRILLED AND SHALL BE 2MM LARGER THAN THE BOLTS DIAMETER U.N.O. HOLES IN BASE PLATES MAY BE 6mm LARGER THAN THE BOLTS DIAMETER U.N.O. ALL BOLTS SHALL HAVE AT LEAST ONE THREAD PROJECTING THROUGH BOTH SIDES OF THE NUT. BOLTS SPACING, EDGE DISTANCES, GAUGE LINES, BEAM COPES ETC, TO CONFORM TO A.I.S.C. STANDARDISED CONNECTIONS UNO. REMOVE SHARP EDGES AND BURRS.

S5. UNLESS OTHERWISE NOTED, ALL WELDS TO BE SP (STRUCTURAL PURPOSE) TO **AS/NZS 1554 PART 1:2000** AND BUTT WELD SHALL BEFULL STRENGTH COMPLETE PENETRATION WELDS WITH E48XX ELECTRODES (480MPa) TO **AS/NZS 1553 PART 1:1995** U.N.O. ALL WELDS SHALL BE GENERAL PURPOSE WELDS UNLESS NOTED OTHERWISE. STRUCTURAL PURPOSE WELDS SHALLDENOTES THUS 'SP' BUTT WELDS WHERE INDICATED IN THE DRAWINGS ARE TO BE COMPLETE PENETRATION BUTT WELDS AS DEFINED IN **AS 1554**. WELDING SYMBOLS TO **AS 1101 PART 3**.

S6. CONCRETE ENCASED STEELWORK SHALL BE WRAPPED WITH 665 MESH AND HAVE A MINIMUM OF 50 COVER UNLESS NOTED OTHERWISE.

S7. HIGH STRENGTH FRICTION GRIP BOLTS, NUTS AND WASHERS SHALL COMPLY WITH THE RELEVANT REQUIREMENTS OF **AS 1252**, SHALL BE INSTALLED IN ACCORDANCE WITH AS 1511 AND SHALL BE TIGHTENED TO THE CORRECT TENSION USING APPROVED LOAD INDICATING WASHERS. CONTACT SURFACES OF ALL HIGH STRENGTH FRICTION GRIP BOLTED CONNECTIONS SHALL BE LEFT UNPAINTED.

S8. THE BUILDER SHALL PROVIDE ALL CLEATS AND DRILL ALL HOLES NECESSARY FOR FIXING STEEL TO STEEL AND TIMBER TO STEEL WHETHER OR NOT DETAILED IN THE DRAWINGS.

S9. STEEL FABRICATOR TO PROVIDE DETAIL SHOP DRAWING TO DESIGN ENGINEER FOR REVIEW AND APPROVAL TO FABRICATE.

S10. PROVIDE LOCKNUT FOR BOLTS IN MOVING PARTS OR PARTS SUBJECTED TO VIBRATION AND FOR VERTICAL BOLTS IN TENSION.

S11. HIGH STRENGTH BOLTED JOINTS SHALL BE IN ACCORDANCE WITH **AS4100**. THE SPECIFIED BOLTS TENSION SHALL BE OBTAINED BT THE USE OF THE "PART TURN" METHOD OF TIGHTENING OR APPROVED TENSION INDICATORS.

S12. WHERE SLOTTED HOLES ARE PROVIDED FOR MOVEMENT CONNECTIONS THE JOINTS SHALL BE FREE TO MOVE IN THE DIRECTION OF THE SLOT. BOLTS SHALL BE PLACED MID SLOT U.N.O. BOLTS HOLES SHALL NOT BE SLOTTED WITHOUT WRITTEN APPROVAL OF THE ENGINEER, U.N.O.

S13. SUBSTITUTIONS FOR STEEL SECTIONS SHOWN ON DRAWINGS SHALL NOT BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

S14. THE CONTRACTOR SHALL PROVIDE ALL CLEATS AND DRILL ALL HOLES NECESSARY FOR FIXING STEEL, TIMBER AND OTHER ELEMENTS TO THE STEEL WHETHER OR NOT DETAILED ON THE STRUCTURAL DRAWINGS.

S15. THE FABRICATION AND ERECTION OF THE STRUCTURAL STEELWORK SHALL BE SUPERVISED BY APPROVED INDEPENDENT WELDING INSPECTION SERVICE TO ENSURE THAT ALL REQUIREMENTS OF THE DESIGN ARE MET. ALL WELDS (AS IDENTIFIED BY ENGINEER) WILL REQUIRE N.D.T TESTING. THIS IS TO BE CARRIED OUT BY A "TELARC" OR INTERNATIONAL ACCREDITED REGISTERED LABORATORY. ULTRASONIC TESTING TO "LEVEL 2" SENSITIVITY (**AS 2207:1994**) IS TO BE BE USED WHERE DIRECTED. LEVEL OF ACCEPTANCE WILL BE "CLASS A" AS DEFINED BY **AS/NZS1554.1:2000.**

STEELWORK CONT.

S16. CONTRACTOR IS TO MAINTAIN A QUALITY ASSURANCE PROGRAM, ENSURING THAT ALL WELDERS AND WELDING PROCEDURES ARE CERTIFIED AS REQUIRED BY **NZS 4711 AND AS/NZS1554.1:2000** AND THAT ALL FABRICATION MEETS THE REQUIREMENTS OF THESE STANDARDS AND OF **NZS 3404:1997**.

S17. ALL STEELWORK IS TO BE SHOP DETAILED, SHOP DRAWINGS ARE TO BE PROVIDED BY THE CONTRACTOR AND REVIEWED BY THE ENGINEER AND ARCHITECT PRIOR TO COMMENCEMENT OF ANY FABRICATION OF THE STEELWORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LAYOUT AND MEMBER DIMENSIONS. PROVIDE:
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ARCHITECT (2 COPIES).

S18. DETAILS OF ERECTION SEQUENCE SHALL BE SUBMITTED TO THE ENGINEER FOR COMMENT PRIOR TO COMMENCEMENT OF ERECTION. THE APPROVED ERECTION SEQUENCE SHALL NOT BE VARIED DURING THE ERECTION WITHOUT WRITTEN COMMENT FROM THE ENGINEER.

S19. COLUMNS AND MULLIONS SHALL HAVE THEIR BASE PLATES FULLY GROUTED IN ACCORDANCE WITH THE SPECIFICATIONS AFTER PLUMBING AND LEVELING ON STEEL PACKERS.

S20. ALL RAFTERS AND BEAMS OVER 6000mm IN LENGTH SHALL BE CAMBERED 5mm FOR EVERY 2000mm OF LENGTH UNLESS NOTED OTHERWISE ON THE DRAWINGS.

S21. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT THE STEELWORK IS NOT DAMAGED OR DISTORTED DURING THE FABRICATION OR GALVANISING PROCESSES.

S22. PURLIN CENTRES NOMINATED ON PLAN ARE MAXIMUM AND MAY NEED TO BE REDUCED TO COMPLY WITH SUPPORT SPACINGS REQUIRED FOR ARCHITECTURAL FINISHES INCLUDING END SPANS. THE FABRICATOR SHALL DETERMINE THE NUMBER OF PURLIN RUNS REQUIRED. THE TOTAL NUMBER OF PURLIN RUNS SHALL BE BASED ON THE WRITTEN INFORMATION CONTAINED ON THE STRUCTURAL AND ARCHITECTURAL DRAWINGS AND THE BUILDING SET-OUT DIMENSIONS SHALL BE OBTAINED FROM THE ARCHITECTURAL DRAWINGS. THE FABRICATOR SHALL PROVIDE ALL TRIMMING ANGLES REQUIRED FOR THE SUPPORT OF GUTTERS, FLASHING AND BOTH EDGE AND END SUPPORTS OF WALL AND ROOF SHEETING.

S23. ALLOW ALL BEAMS TO BE FLY BRACED AT 5000 CTS UNLESS NOTED. OTHERWISE ALLOW BRACES TO EACH SIDE OF TRANSFER BEAMS.

S24. REFER TO ARCHITECTS DRAWINGS FOR ANY ADDITIONAL INCIDENTAL AND SECONDARY STEELWORK REQUIRED NOT SHOWN ON STRUCTURAL DRAWINGS.

S25. UNLESS SHOWN OTHERWISE ON THE DRAWINGS, ALL CONNECTIONS SHALL BE IN ACCORDANCE WITH THE FOLLOWING MIN REQUIREMENTS:
(a) ALL WELDS SHALL BE FULL STRENGTH BUTT WELDED (FSBW).
(b) ALL BOLTS SHALL BE M20-8.8/S, WITH A MIN. OF 2 BOLTS PER CONNECTION.
(c) ALL GUSSET, STIFFENERS AND CLEAT PLATES SHALL BE 10mm THICK.
(d) ALL CAP PLATES SHALL BE 12mm THICK.
(e) ALL BASE PLATE SHALL BE 20mm THICK.
(f) ALL END/SEAL PLATES SHALL BE 6mm THICK.
(g) ALL BOLTS SHALL HAVE AT LEAST THREE THREAD PROJECTING THROUGH BOLT SIDES OF NUT.

S26. EDGE DISTANCE FOR BOLTS: M16 = 30mm M20 = 35mm

S27. ALL HOLES SHALL BE DRILLED AND SHALL BE 2mm LARGER THAN THE BOLT DIAMETER U.N.O. HOLES IN BASE PLATES MAY BE 6mm LARGER THAN THE BOLT DIAMETER U.N.O.

S28. ALL STEELWORK SHALL COMPLY BELOW:
(a) STRAIGHTNESS OF MEMBERS AFTER FABRICATION AND BEFORE ERECTION U.N.O SHALL NOT DEVIATE MORE THAN:
STRUTS AND COLUMNS = L/1000
OTHER MEMBERS = L/600
(b) LENGTH SHALL NOT DEVIATE FROM THE TRUE LENGTH BY:
STRUT WITH END BEARING = +-1mm
OTHER MEMBERS UP TO L = 9.0m = +0mm, -3mm
OTHER MEMBERS OVER L = 9.0m = +0mm, -5mm

S29. STRUCTURAL STEELWORK SHALL HAVE THE SURFACE TREATMENT IN ACCORDANCE WITH THE FOLLOWING SPECIFICATION AND **AS/NZS 2312: PART 1 AND PART 2-2014.**
ROUND OFF ALL ROUGH WELDS, SHARP EDGES AND REMOVE WELD SPLATTER. REMOVE GREASE, OIL AND CONTAMINANTS IN ACCORDANCE WITH **AS1627.1**. RINSE SURFACE THOROUGHLY TO REMOVE ACID OR ALKALINE CONTAMINATION. ABRASIVE BLAST CLEAN TO A MINIMUM OF **AS1627.4 CLASS 2.5**. REMOVE ALL DUST BY BRUSHING OR VACUUM CLEANING. APPLY PRODUCT BEFORE ANY SURFACE DETERIORATION OCCURS.

PAINT SYSTEM - RESENE PUR 5			
Elements:	Surface exposed to weather only		
Surface cleaning:	SA 2.5		
Durability:	15-25 years		
	1st Coat	2nd Coat	3rd Coat
Material Product Code: Data Sheet:	Zincilate 10/11 or ArmourZinc 120	Armourcote 510	Uracryl 403
Coating thickness DFT (microns)	**75**	**200**	**50**
Total	**325**		

WFT*- IS THE THICKNESS OF WET PAINT REQUIRED TO APPLIED TO THE SURFACE TO ACHIEVE THE SPECIFIED DRY FILM THICKNESS. NOTE: CONCRETE ENCASED STEELWORK SHALL BE LEFT UNPAINTED. FINISHING PAINT COATING COLOUR SHALL BE ACCORDANCE TO CLIENT AND ARCHITECTS SPECIFICATION IF NOT SPECIFIED IN THE SCOPE OF WORK. PERMIT REPRESENTATIVES OF DULUX TO INSPECT THE WORK IN PROGRESS AND TAKE SAMPLES OF THEIR PRODUCTS FROM SITE IF REQUESTED AND ISSUE COMPLIANCE CERTIFICATE AFTER FINAL INSPECTION. ABOVE TABLE IS ONLY FOR REFERENCE. SEEK PROFESSIONAL ADVISE FROM PAINT MANUFACTURER FOR RELEVANT PAINT SYSTEM TO TO BE APPLIED FOR THIS PRODUCT. PAINT SYSTEM MAY SUBJECT TO CHANGE DEPENDING ON SITE CORROSIVELY ZONE.

STEELWORK CONT.

S30. ALL STRUCTURAL STEEL WORK SHALL BE HOT DIPPED GALVANIZED TO CX CATEGORY TO AS2312.2-2014 WITH MINIMUM COATING 900g/m2(125 MICRON) AND APPLY SI DUPLEX SYSTEM TO INCREASE LIFE OF FIRST MAINTENANCE.ALL OTHER STEEL WORK, FASTENERS, BOLTS, WASHERS, NUTS, PLATES AND CONNECTIONS SHALL BE TYPE 316 STAINLESS STEEL U.N.O.

S31. UNPAINTED STEELWORK SHALL BE FREE FROM SCALE. FINISH: AS PER ARCHITECTURAL SPECIFICATION.

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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NRW MACALLAN (FIJI) LTD CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

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Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	

QUALITY ASSURANCE
_____ _____
DIRECTOR DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

Drawing Title:
GENERAL NOTES (SHEET 2 OF 4)

Job No: 22507209

Drawing No: G02

Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A

GENERAL NOTES - 3

TIMBER:

T1. ALL TIMBER DESIGN, CONSTRUCTION AND MATERIALS SHALL BE IN ACCORDANCE WITH **AS 1720 AND AS 1684**. ALL TIMBER FRAMING SHALL BE CONSTRUCTED IN GENERAL ACCORDANCE WITH **AS 1684** OR THE "TIMBER FRAMING MANUAL" THE BY TIMBER PROMOTION COUNCIL.

T2. STRUCTURAL TIMBER AS SHOWN ON THE ENGINEER'S DRAWINGS SHALL HAVE A MINIMUM STRESS GRADE AS SHOWN BELOW U.N.O.
F17 KILN DRIED HARDWOOD (KDHW)
F8 UNSEASONED HARDWOOD (HW)
F7 OREGON (DOUGLAS FIR UNSEASONED)
F5 SEASONED RADIATA PINE (KDRP)
LVL LAMINATED VENEER LUMBER (HYSPAN)
MGP MACHINE GRADED RADIATA PINE (MGP12)

T2. ALL TIMBER MEMBERS SHALL BE GRADED UNDER THE AUTHORITY OF A RECOGNIZED QUALITY ASSURANCE PROGRAM VIZ, CYPRESS QUALITY ASSURANCE PROGRAM, AUST. HARDWOOD QUALITY CONTROL, PINE QUALITY ASSURANCE PROGRAM.

T4. EXCEPT AS PROVIDED BELOW, ALL TIMBER USED FOR STRUCTURAL PURPOSES SHALL BE BRANDED AND THE BRAND SHALL CONTAIN THE FOLLOWING INFORMATION: -
- THE GRADE IN TERMS OF AN "F" RATING e.g. F11.
- THE METHOD OF GRADING IF OTHER THAN VISUAL STRESS GRADING e.g. MACHINE STRESS GRADING "MSG" OR MACHINE PROOF GRADING "PROOF" OR "PG".
- IF SEASONED, THE WORD "SEASONED" OR THE LETTER "S".
- A NAME, INITIALS, LOGO OR NUMBER WHICH INDICATES THE SOURCE OF GRADING.
- THE CERTIFICATION TRADE MARK OF THE RELEVANT ASSURANCE PROGRAM.

T5. PLYWOOD USED FOR STRUCTURAL PURPOSES SHALL BE BRANDED WITH THE PLYWOOD ASSOCIATION OF AUSTRALIA (P.A.A.) TEST MARK AND MANUFACTURED IN ACCORDANCE WITH THE RELEVANT AUSTRALIAN STANDARDS AND UNDER THE P.A.A. QUALITY CONTROL PROGRAM.

T6. TIMBER SIZES NOT CALLED UP SHALL BE IN ACCORDANCE WITH AS 1684 OR THE ARCHITECT'S DRAWINGS. ANY DISCREPANCY SHALL BE REFERRED TO THE ARCHITECT.

T7. ALL BOLTS, NUTS AND WASHERS IN TIMBER CONNECTIONS SHALL BE GALVANIZED MILD STEEL IN ACCORDANCE WITH AS 1214. HOLES FOR BOLTED CONNECTIONS SHALL BE PRE-BORED TO NOT MORE THAN 10 % GREATER THAN THE BOLT DIAMETER. ALL BOLTS WHICH ARE NOT SPECIFIED AS COACH BOLTS SHALL BE HEXAGONAL HEAD BOLTS AND SHALL BE FITTED WITH WASHERS OF THE FOLLOWING MINIMUM SIZES TO BOLT HEADS AND NUTS WHICH ARE IN CONTACT WITH TIMBER: MINIMUM TIMBER EDGE DISTANCE AND BOLT SPACING TO BE 5 x BOLT DIAMETER. NO KNOTS OR DEFECTS SHALL OCCUR WITHIN 150mm OF BOLT GROUP OR CONNECTORS. WHERE POSSIBLE, RE-TIGHTEN BOLTS AFTER 6 WEEKS AND AGAIN AT 12 MONTHS.

T8. MAKE GOOD PRESERVATIVE TREATMENT WHERE CHECKOUTS, HOLES AND CUTS EXPOSE UNTREATED TIMBER.

T9 ALL EXTERNAL TIMBER SHALL BE EITHER HARDWOOD DURABILITY CLASS 1 OR 2 OR IMPREGNATED PINE GRADE F7, PRESSURE TREATED TO AS 1604 (FOR THE APPROPRIATE HAZARD LEVEL) AND RE-DRIED PRIOR TO USE. SUPPLEMENTARY TREATMENT SHALL BE APPLIED TO ALL CUT SURFACES. SUPPLY SUPPORTING DOCUMENTATION FOR REPRESENTATIVE TREATMENT. ONLY TIMBER OF HIGH DURABILITY IS PERMITTED IN CONTACT WITH SOIL.

T10. ALL TIMBER FRAMEWORK SHALL BE ADEQUATELY TIED TO RESIST UPLIFT AND RACKING FORCES IN ACCORDANCE WITH AS 1684. ALL MEMBERS BETWEEN THE ROOF AND THE FOUNDATIONS ARE TO BE ADEQUATELY CONNECTED TO EACH OTHER.

T11. PLYWOOD AND HARDBOARD PANEL WALL BRACING AND TENSIONED METAL CROSS BRACING TO BE COMPLY WITH AS 1684. STUDS ARE NOT BE NOTCHED FOR BRACING.ROOF BRACING SHALL BE COMPLY WITH AS 1684.

T12. RAFTERS OR FLOOR JOISTS BUTTING INTO SIDES OF THE TIMBER BEAM ARE TO CONNECTED USING `PRYDA' FRAMING BRACKETS OR `PRYDA' MULTIGRIPS TO EACH SIDE OF THE MEMBER.

T13. WHERE ROOF LIGHTS OR OTHER INSTALLATIONS PENETRATE THE ROOF FRAMING, TRIM EACH SIDE OF THE OPENING WITH DOUBLE RAFTERS TO MAINTAIN AVERAGE RAFTER SPACING. USE TRIMMER RAFTERS TO ADJACENT SIDES TO TRANSFER LOADS TO CONTINUOUS RAFTERS.

T14. TIMBER POSTS SUPPORTING PERGOLA FRAMING ARE TO BE SUPPORTED 50mm CLEAR OFF THE FINISHED GROUND LEVEL ON STANDARD GALVANIZED STEEL STIRRUPS.

T15. WHERE ISOLATED INTERNAL POSTS ARE SUPPORTED ON A SLAB-ON-GROUND, INVERTED STEEL 'T' BRACKETS ARE TO BE FIXED TO THE SLAB WITH EXPANSION ANCHORS, AND THE POST FIXED TO THE BRACKET WITH 2-M10 COACH BOLTS. PROVIDE FLAT STEEL WASHERS UNDER NUTS.

T16. UNLESS NOTED OTHERWISE PROVIDE DOUBLE STUDS TO EACH END OF LINTELS AND BEAMS. DOUBLE STUDS TO BE OF MINIMUM SECTION 2/90x45 LAMINATED TOGETHER WITH 90mm NAILS AT 600mm CTS IN ACCORDANCE WITH THE TIMBER FRAMING MANUAL AND AS 1684. LESSER MEMBER SIZES MAY ONLY BE USED IF COMPLIANT WITH THESE CODES. CONTINUE DOUBLE STUDS TO LOWER LEVELS TO CARRY MAJOR POINT LOADS TO FOUNDATIONS.

TIMBER CONT.:

T17. ALL TIMBER SHALL BE PROTECTED FROM WEATHER BEFORE AND AFTER FABRICATION. TIMBER IS TO BE KEPT AS DRY AS POSSIBLE. INTERNAL LININGS ARE NOT TO BE FIXED IN PLACE UNTIL TIMBER FRAMING IS SUITABLY DRY. ALL REMEDIAL WORK THAT IS CAUSED BY USING WET TIMBER, IS THE CONTRACTORS RESPONSIBILITY TO FIX.

T18. ALL TIMBER SHALL BE NO. 1 FRAMING GRADE AS DEFINED IN THE NATIONAL GRADING RULES FOR FIJI TIMBERS.

T19. ALL STRUCTURAL FRAMING TIMBER SHALL BE FIJI - C-SELECT CORRESPONDING TO STRESS GRADE F7/F8 SEASONED AND PRESERVATIVE TREATED ACCORDING TO FIJI FORESTRY DEPARTMENT RECOMMENDATIONS FOR RESPECTIVE END USE.

T20. ALL BOLTS SHALL BE GRADE 8.8/S OR 4.6/S AS SPECIFIED TO **AS/NZ 1252 (1996), AS111 (2000)**, AS **1112(2000) AND AS 1237 (2002),** HOLES AND FIXING FOR TIMBERS, ARE TO BE SHOP FABRICATED INTO STEEL WHERE SHOWN OR IMPLIED ON ARCHITECTURAL DRAWINGS.

BOLTS SIZE:	SQ. WASHER	ROUND WASHER:
M10 OR M1240 X	40 X 3.0	40Ø X 3.0
M16 OR M2050 X	50 X 3.0	50Ø X 4.0

T21. ALL WALL NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE TIMBER FRAMED.

T22. ALL STUDS SHALL BE NOGGED AT 600mm CRS. REFER TO ARCHITECTURAL DRAWINGS FOR ACOUSTIC AND FIRE RATING REQUIREMENTS FOR WALL.

T23. TIMBER TO BE KEPT AS DRY AS POSSIBLE. INTERNAL LINING ARE NOT TO BE FIXED IN PLACED UNTIL TIMBER FRAMING IS SUITABLE DRY. ALL REMEDIAL WORKS THAT IS CAUSED BY USING WET TIMBER, IS THE CONTRACTORS RESPONSIBILITIES TO FIX.

T24. ALL 2-100x50 AND 2-150x50 TIMBER POSTS TO BE FIXED DOWN TO CONCRETE FLOORING WITH A MINIMUM OF 2-M12 BOLTS AND 2 No. 6KN NAIL PLATES STRAPS.

T25. ALL FRAMING AND STRUCTURAL TIMBER SHALL BE IN ACCORDANCE WITH THE FOLLOWING STANDARDS:

AS/NZS 2269.0:2012	STRUCTURAL PLY
AS/NZS 2269.1:2012	STRUCTURAL PLY
NZS 3602:2003	TIMBER PRODUCTS IN BUILDING
NZS 3603:1993	TIMBER STRUCTURAL STANDARDS
NZS 3604:2011	TIMBER FRAMED BUILDING
NZS 3605:2001	TIMBER PILES AND POLES FOR USE IN BUILDING.

T26. ALL TIMBER SHALL BE TREADED:
H3.1 - ALL TIMBER INCLUDING LINING & CLADDING UNO.
H3.2 - ALL EXPOSED JOISTS & RAFTERS
H5 - ALL PILES (NOTED H5 NZ=H4 FJ)

T27. ALL WALL PLATES, BEARERS, DOOR & WINDOWS FRAMES SHALL BE DENSE LOCAL HARDWOOD OR BE STRESS GRADE F8 MINIMUM. ALL OTHER TIMBER STRESS F7. NON-STRUCTURAL TIMBER STRESS GRADE F5.

T28. TIMBER FASTENERS SHALL BE IN ACCORDANCE TO NZS 3604:2011 SECTION 4 DURABILITY, ZONED IN TABLE2-4 APPLIED STRICTLY AS SPECIFIED IN THE CONTRACT DOCUMENTS AND DRAWINGS. ALL NAILS SPIKES UNLESS OTHERWISE SHOWN SHALL BE HOT DIPPED GALVANIZED. ALL SHELTERED EXPOSED FASTENERS TO BE TYPE 304 OR TYPE 316 STAINLESS STEEL.

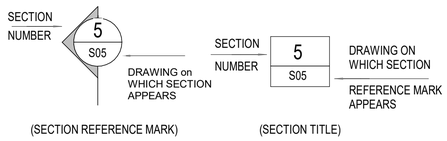
T29. THE BUILDER SHALL WHERE NECESSARY PROVIDE SHOP DRAWINGS FOR THE FABRICATION OF TRUSS AND WALL PANELS. FABRICATION SHALL NOT COMMENCE UNTIL APPROVAL HAS BEEN ISSUED. APPROVAL DOES NOT INCLUDE DIMENSIONS.

T30. END AND EDGES DISTANCE FOR BOLTS WHERE NOT SPECIFIED SHALL BE IN ACCORDANCE WITH THE PROVISION OF AS 1720.1

T31. PROVIDE MALTHOID AT 1.0 MAXIMUM CENTERS FOR ALL TIMBER CONTACT WITH CONCRETE. WHERE TIMBER COMES IN CONTACT CONCRETE, PROVIDE A SEPARATION LAYER OF MALTHOID D.P.C BETWEEN THE TIMBER AND CONCRETE SURFACES.

T32. TIMBER FRAMING TO BE FIXED TO STEELWORK WITH M12 BOLTS AT 800 MAX. C/C UNO.

DESIGNATION OF CROSS SECTIONS



CONCRETE BLOCK MASONRY

CB1. ALL MASONRY UNITS SHALL BE MANUFACTURED IN ACCORDANCE WITH **AS/NZS 4455: 1997** ALL MASONRY WALLS SHELL BE CONSTRUCTION TO THE REQUIREMENTS OF **NZS 4210: 2001** FOR GRADE B. CHARACTERISTIC UNCONFINED COMPRESSIVE STRENGTH OF 12MPa.

CB2. ALL CELLS IN BLOCK WORK TO BE FILLED WITH GROUT, 25 MPA MIN STRENGTH AT 28 DAYS. MORTOR ADMIXTURES SHALL NOT BE USED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

CB3. ALL GROUT SHALL BE CARRIED OUT USING THE HIGH LIFT GROUT METHOD. UNO.

CB4. ALL REINFORCEMENT STARTERS TO BE ACCURATELY LOCATED, TO SUIT BLOCKWALL MODULES. BENDING REINFORCEMENT AT SLAB LEVEL TO ALIGN WITH BLOCKWALL CELLS WILL NOT BE PERMITTED.

CB5. ALL FOR MAKING ALL NECESSARY PENETRATIONS AND EMBEDMENT OF ALL NECESSARY CONDUITS AND FIXING IN BLOCK WALLS. PENETRATION SHALL BE FORMED, RATHER THAN CUT.

CB6. ALL EXPOSED BLOCK WALL RESTRAINT BRACKETS TO BE FIRE RATED TO THE EQUIVALENT FIRE RATING REQUIRED FOR THE FLOOR.

CB7. PROVIDE BLOCK WALL CONTROL JOINTS AT 6.0M MAXIMUM CENTERS ALONG LENGTH OF WALLS AND 4.0M MAX.CENTERS VERTICALLY.

CB8. WHERE NOT SHOWN BLOCKWALL TO BE REINFORCED VERTICALLY AT CORNERS, SIDE OF OPENINGS, END OF WALL, INTERSECTIONS AND AT 400 CRS WITH HD16'S AND HORIZONTALLY AT TOP OF & BOTTOM OF OPENING AND 200 CRS MAX. WITH HD16'S.

CB9. MORTAR JOINTS SHALL BE 10mm THICK AND HAVE A MAXIMUM TOOLED DEPTH OF 3mm U.N.O.

CB10. NO CHASES SHALL BE CUT INTO LOAD-BEARING MASONRY WITHOUT THE APPROVAL OF THE ENGINEER.

CB11. CLEAN OUT HOLES SHALL BE PROVIDED AT THE BASE OF ALL CORES OR CAVITIES WHICH ARE TO BE GROUTED OR FILLED.

CB12. GROUT FOR BOND BEAMS, CORE FILLING OR CAVITY FILLING SHALL COMPRISE OF 1 PART CEMENT, 0.25 PART LIME, 3 PARTS 10mm AGGREGATE UNLESS NOTED OTHERWISE ON THE DRAWINGS. MAXIMUM SLUMP TO BE 230mm.

CB13. REINFORCING STEEL SHALL BE SECURELY FIXED IN POSITION BEFORE GROUTING.

CB14. GROUT SHALL BE THOROUGHLY COMPACTED USING A PLAIN BAR.

CB15. MASONRY IS NOT TO BE ERECTED ON FLOORS UNTIL FORMWORK AND FALSEWORK SYSTEMS PROVIDING SUPPORT HAVE BEEN REMOVED.

CB16. THE DURABILITY REQUIREMENTS FOR MASONRY SHALL BE AS PER SEVERE ENVIRONMENT WITH M4 (MORTAR), R4 (BUILT IN COMPONENT) AND 30mm MIN COVER TO REINFORCEMENT AND TENDONS IN GROUTED CAVITIES OR CORES.

GLAZING/BUILDING ENVELOPE

G1. GENERAL: REFER TO THE APPLICABLE FIJI STANDARD FORM OF BUILDING CONTRACT AND THE PRELIMINARY & GENERAL CLAUSES WHICH WILL ALSO APPLY TO THIS WORK. THE FOLLOWING STANDARDS SHALL BE FORM PART OF THIS SPECIFICATION: **NZSS SR2, AS 1170 PART 2: 2011, AS 2014/2048 AS 1288 PART 1:1979** ``GLASS INSTALLATION CODE"

G2. STRENGTH CERTIFICATION: THE METAL WINDOWS & DOOR SUPPLIER AND THE GLAZIER WILL BE REQUIRED TO CERTIFY THAT THE METAL DOOR & WINDOW FRAMING & GLAZING SYSTEM IS CAPABLE OF WITHSTANDING THE PERMISSIBLE WINDSPEED OF 70m/sec & LOADINGS IN ACCORDANCE WITH STANDARDS. THE HEIGHT OF THE BUILDING SHOULD ALSO BE CONSIDERED. THE GLAZING SUPPLIES/DESIGN SHALL CONSIDER OF ALL LOADING SITUATION IN **AS/NZS 1170.2:2011**

G3. ALL WINDOWS/DOORS FRAMING, GLAZING & FIXING TO BE ABLE TO RESIST ULTIMATE LIMIT STATE DESIGN WINDSPEED OF 70 m/s & SLS WINDSPEED OF 45 m/s. ALL DESIGN WIND PRESSURES SHALL BE OBTAINED USING **AS/NZS 1170.2:2011.** GLAZING TO WINDSTAND DEBRIS IMPACT AS WELL AS WIND PRESSURE AS PER **AS/NZS 1170.2:2011**.

G4. ALL GLASS DESIGN & INSTALLATION TO BE IN ACCORDANCE TO **AS/NZS 4223 PART 1-4.**

G5. THE CONTRACTOR TO PROVIDE FULL SET OF CALCULATIONS, SHOP DRAWINGS & CERTIFICATION TO ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

G6. ALL ROLLER SHUTTER DOORS TO BE CYCLONE RATED TO ULTIMATE WINDSPEED OF 70M/S. CONTRACTOR TO PROVIDE CERTIFICATION OF ROLLER SHUTTER DOOR PRIOR TO INSTALLATION.

G7. SEISMIC DESIGN TO BE AS PER **NZS1170.5:2004** USING ZONE FACTOR = 0.27

DRAFTING ABBREVIATIONS

ALT.	ALTERNATE	MIN.	MINIMUM
APPROX.	APPROXIMATE	MS	MILD STEEL
ADDN	ADDITIONAL	{N}	NEW
B	BOTTOM	N.D.T	NON-DESTRUCTIVE TESTING
BLK	BLOCKWALL	NF	NEAR FACE
B.S	BOTH SIDES	NOM.	NOMINAL
BW	BOTHWAYS	No.	NUMBER
C	CENTRE	NTS	NOT TO SCALE
C/L	CENTRE LINE	O/A	OUTSIDE OVERALL
C/C	CENTRE TO CENTRE	O.D	OUTSIDE DIAMETER
C/S	COURSES	PL.	PLATE
C.A.R	COVER ALL AROUND	PC	PRECAST CONCRETE
CHS	CIRCULAR HOLLOW SECTION	PSC	PRESTRESSED CONCRETE
C.J.	CONTROL JOINT	R	PLAIN BAR GRADE 300
COL	COLUMN	RC	REINFORCED CONCRETE
CONC.	CONCRETE	REINF	REINFORCEMENT
CONN.	CONNECTION	RHS	RECTANGULAR HOLLOW
C.O.S.	CHECK ON SITE	SECT	SECTION
CVR	COVER	SHT	SHEET
CRS	CENTRES	SPEC	SPECIFICATION
D	DEFORMED BAR GRADE 300	S.E.D	SMALL END DIAMETER
db	BAR DIAMETER	RSC	ROLLED STEEL CHANNEL
DET	DETAIL	RSJ	ROLLED STEEL JOIST
DIA	DIAMETER	STIFF	STIFFENER
D.J.	DOWELED JOINT	SIM	SIMILAR
DPC	DAMP PROOF COURSE	SJ	SAWCUT JOINT
DWG/DRG	DRAWING	STG	STAGGER
EGL	EXISTING GROUND LEVEL	STIR	STIRRUP
EXTG	EXISTING	STA	STARTER
EF	EACH FACE	SHS	SQUARE HOLLOW SECTION
EW	EACH WAY	STR, STA	STARTER
EL	ELEVATION	SYM.	SYMMETRICAL
EX	OUT OF	T	TOP
FF	FAR FACE	TBC	TO BE CONFIRM
FFL	FINISHED FLOOR LEVEL	TFB	TAPER FLANGE BEAM
FGL	FINISHED GROUND LEVEL	THK	THICK
FL	FLAT	TOC	TOP OF CONCRETE
GALV'D.	GALVANISED	TOS	TOP OF STEEL
G.L	GROUND LEVEL	TRM	TRIMMER
G.P.C.	GROUT PROOF COURSE	TYP	TYPICAL
HORZ.	HORIZONTAL	UB	UNIVERSAL BEAM
H.D.	HOLDING DOWN (BOLT)	UC	UNIVERSAL COLUMN
HD	HIGH DUCTILE BAR (500E)	UNO	UNLESS NOTED OR SHOWN OTHERWISE
I.D	INSIDE DIAMETER	U/S	UNDERSIDE
I.L	INVERT LEVEL	VERT.	VERTICAL
I.P	INTERSECTION POINT	L	SINGLE RSA
KJ	KEYED JOINT	VJ	VERTICAL JOINT
LAR	LAP AT RANDOM	VL	VARYING LENGHT
⅄L	DOUBLE RSA (BACK TO BACK)	VERT.	VERTICAL
LG	LONG		
MAX.	MAXIMUM		

ALUMINUM FABRICATIONS.

A1. DESIGN, FABRICATION & INSTALLATION IN ACCORDANCE WITH CSA S157.

A2. PERFORM WELDING OF ALUMINUM IN ACCORDANCE WITH REQUIREMENTS OF CSA W59.2 AND CSA S244, FABRICATION TO BE CERTIFIED TO DIVISION

A3. ALUMINUM TO CAS/CAN 3-S157, 6061-T6 ALUMINUM ALLOY.

A4. BOLTS AND ANCHOR BOLTS STAINLESS STEEL, ASTM 316 C/W ISOLATION WASHERS

A5. ISOLATE ALUMINUM FROM FOLLOWING COMPONENTS, BY MEANS OF, 2 COATS OF ALKALI RESISTANT BITUMINOUS PAINT.

1. DISSIMILAR METALS EXCEPT STAINLESS STEEL, GALVANIZED STEEL, ZINC, OR WHITE BRONZE OF SMALL AREA.
2. CONCRETE. MORTAR AND MASONRY

PRECAST.

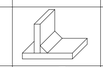
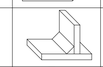
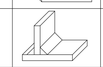
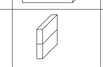
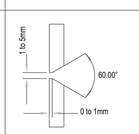
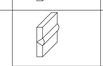
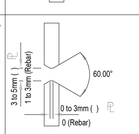
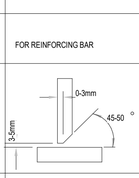
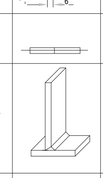
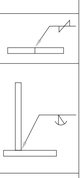
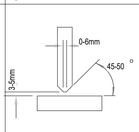
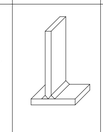
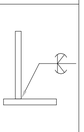
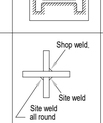
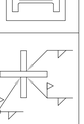
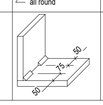
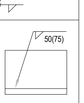
P1. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ALL PRECAST ELEMENTS SUCH AS SLAB & WALLS TO ENGINEER FOR REVIEW AND APPROVAL

P2. CONTRACTOR SHALL PROVIDE PS1 DESIGN CERTIFICATE VIA A REGISTERED ENGINEER OF FIE FOR ALL UNISPAN FLOORING.

P3. THE EQUAL ANGLE SEATING, INCLUDING ANCHOR BOLTS FOR UNISPAN SHALL BE APPLIED WITH FIRE RESISTING FOAM FOLLOWING INSTALLATION WITH A FRC RATING OF 60/60/60.

P4. DESIGN REQUIREMENTS FOR PRECAST UNISPAN FLOORING
1. DESIGN LOADS - SDL = 0.75 KPa
LL = 2.0 KPa
2. DURABILITY CLASS - CLASS B2 (NZS 3101 - 2006)
3. FIRE RESISTANCE - FRL = 60/60/60

WELDING SYMBOLS

FORM OF TEXT	NOTES	SKETCH OF WELD	SYMBOLIC REPRESENTATION
FILLET	WHEN THE SYMBOL IS BELOW THE REFERENCE LINE. IT REFERS TO A WELD ON THE `ARROW SIDE" OF THE JOINT.		
FILLET	WHEN THE SYMBOL IS ABOVE THE REFERENCE LINE IT REFERS TO A WELD ON THE `OTHER SIDE" OF THE JOINT.		
DOUBLE FILLET	WHEN THE SYMBOL IS BOTH SIDES OF THE REFERENCE LINE IT REFERS TO WELDS ON "BOTH SIDES" OF THE JOINT.		
SQUARE BUTT	WHEN THIS TYPE OF WELD IS USED COVERING NOTES WILL BE GIVEN.		
SINGLE V BUTT			
SINGLE V BUTT WITH SEALING RUN.			
DOUBLE V BUTT			
SIZE & LENGTH OF FILLET WELDS	SIZE OF WELDS (MM) BEFORE SYMBOL, LENGTH OF WELD (MM) AFTER SYMBOL.		
FILLET WITH UNEQUAL LEG LENGTH	VERTICAL LEG LENGTH (MM) GIVEN FIRST,HORIZONTAL LEG LENGTH (MM) FOLLOWS.		
FLASH BUTT	FOR REINFORCING BAR		
SINGLE BEVEL BUTT			
DOUBLE BEVEL BUTT			
SHOP WELD ALL ROUND	A CIRCLE AT THE JOINT IN THE REFERENCE LINE INDICATES A CONTINUOUS SHOP WELD ALL AROUND THE JOINT.		
SHOP AND SITE WELD	SITE WELD INDICATED BY FLAG AT THE JOINT IN THE REFERENCE LINE. WITH THE ADDITION OF AN OUTER CIRCLE AT JOINT IN REFERENCE LINE INDICATES A CONTINUOUS SITE WELD ALL ROUND.		
INTER-MITTENT WELDS: LENGTH AND SPACING	WELDED LENGTH GIVEN BY UNBRACKETED NUMBER (MM) SPACE BETWEEN ELD (MM) GIVEN BY BCKETED NUMBER EG 0(75)		

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

Client

fmc PACIFIC

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE	
Checked: RC	Date: 08/14/25	CAD File:	
Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	

QUALITY ASSURANCE
DIRECTOR DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

Job No: 22507209

Drawing No: G03

Drawing Title:
GENERAL NOTES (SHEET 3 OF 4)

Scales:
A1 AS SHOWN
A3 HALF SHOWN

REVISION A

COMPOSITE STEEL NOTES

CS1. ALL MATERIALS, WORKMANSHIP, FABRICATION AND ERECTION SHALL COMPLY WITH THE REQUIREMENTS AS4100, AS3600, AS2327.1. REFER ALSO TO THE CONCRETE AND STEEL NOTES

CS2. SHEAR STUD SHALL BE HEADED SHEAR STUD CONNECTORS CONFORMING TO AS2327.1. WELDING OF THE STUDS SHALL BE THROUGH THE DECKING IN ACCORDANCE WITH AS1554.2. THE SHEARS STUDS SHALL BE 19mm DIAMETER OF MINIMUM TENSILE STRENGTH OF 410MPa. LENGTH OF THE STUDS SHALL BE 25mm LESS THAN THE SLAB THICKNESS UNLESS NOTED OTHERWISE

CS3. THE FLOOR IS TO BE CONSTRUCTED WITHOUT PROPS UNLESS PROPPING IS CLEARLY SHOWN OR NOTED IN THE DRAWINGS

CS4. CONSTRUCTION LOADING SHALL NOT EXCEED LOADING AS PER APPENDIX OF AS2327.1

CS5. WHERE NUMBER OF STUDS SPECIFIED IN THE BEAM SCHEDULE, THE STUDS SHALL BE ARRANGED AS UNIFORMLY AS POSSIBLE. THE STUDS SHALL BE ARRANGED IN SINGLE ROW, CENTRAL TO THE BEAM FLANGE (U.N.O)

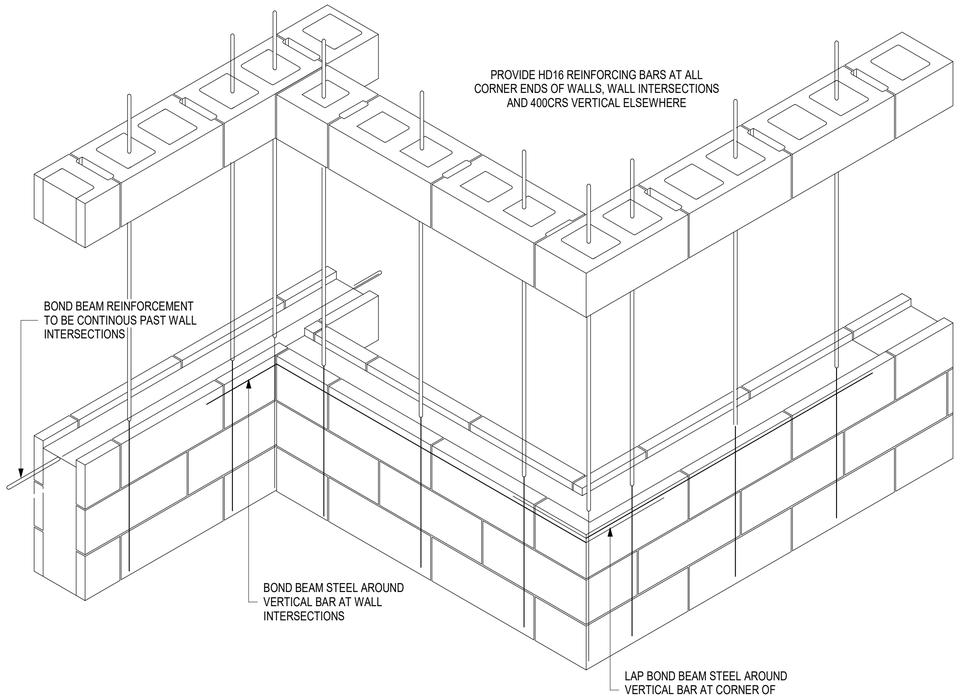
CS6. STUD WELDING SHALL BE BEND TESTED TO AS1554.2 AT THE DIRECTION OF THE ENGINEER OR THE WELDING INSPECTOR. STUDS SHALL PASS THE TEST IF BENT 15 FROM VERTICAL WITHOUT WELD CRACKING OR FAILING. STUDS THAT PASS THE TEST MAY BE LEFT IN THE BENT CONDITION. STUDS THAT FAIL SHALL BE REPLACED.

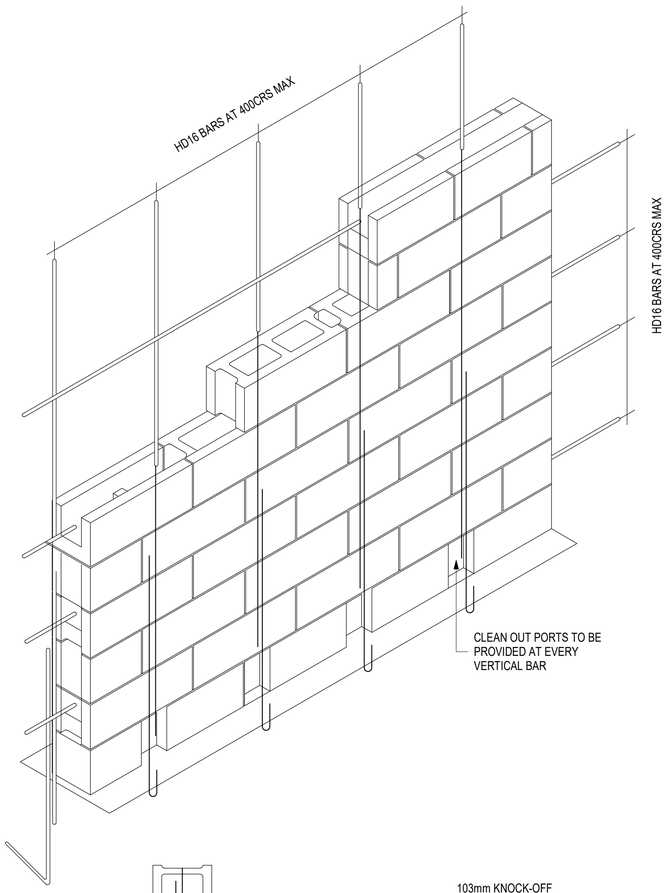
CS7. FLOOR BEAM CAMBERS SHALL BE AS SCHEDULED, THESE CAMBERS WILL GIVE LEVEL FLOOR UNDER THE FULL WEIGHT OF THE WET CONCRETE.

CS8. METAL DECK SHALL BE 1.0mm BMT GRADE 550 Z450 BONDEK

CS9. METAL DECK SHALL BE CONTINOUS AS MUCH AS POSSIBLE ENDS SHALL HAVE MINIMUM BEARING WIDTH OF 50mm. PROVIDE MINIMUM 50mm WIDE TEMPORARY TIMBER PLATE AT CONCRETE FACES FOR THE TRAY SUPPPORT.

CS10. THE METAL DECK SHALL NOT BE PROPPED, EDGES SHALL BE FORMED USING FORMWORK SUSPENDED FROM THE STEEL WORK

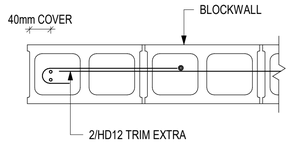


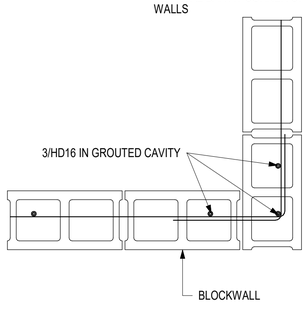


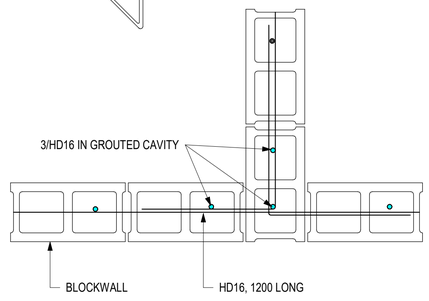
MASONRY LINTEL SCHEDULE

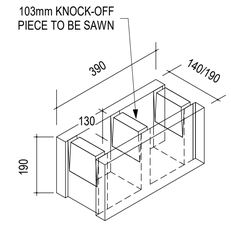
MASONRY LINTEL SCHEDULE		
CLEAR SPAN	LINTEL SIZE	
	TYPE	REINFORCEMENT
0 - 800	1	1 - HD12 TOP AND BOTTOM
800 - 1200	1	1 - HD12 TOP AND BOTTOM
1200-1800	2	2 - HD12 TOP AND BOTTOM
1800-2400	2	2 - HD12 TOP AND BOTTOM
2400-3000	2	2 - HD16 TOP AND BOTTOM
3000-3600	3	2 - HD12 TOP AND BOTTOM
3600-4200	3	2 - HD16 TOP AND BOTTOM
4200-4800	3	2 - HD20 TOP AND BOTTOM

NOTE:
1. 100 MIN. END BEARING FOR LINTELS UP TO 1800 SPAN. 200 MIN END BEARING FOR LINTELS OVER 1800 SPAN.
2. TABLE BASED ON LINTELS SUPPORTING MASONRY ONLY AND FULL ARCING OF MASONRY
3. REFER TO ARCHITECTS DRAWINGS FOR LINTELS LOCATIONS









FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

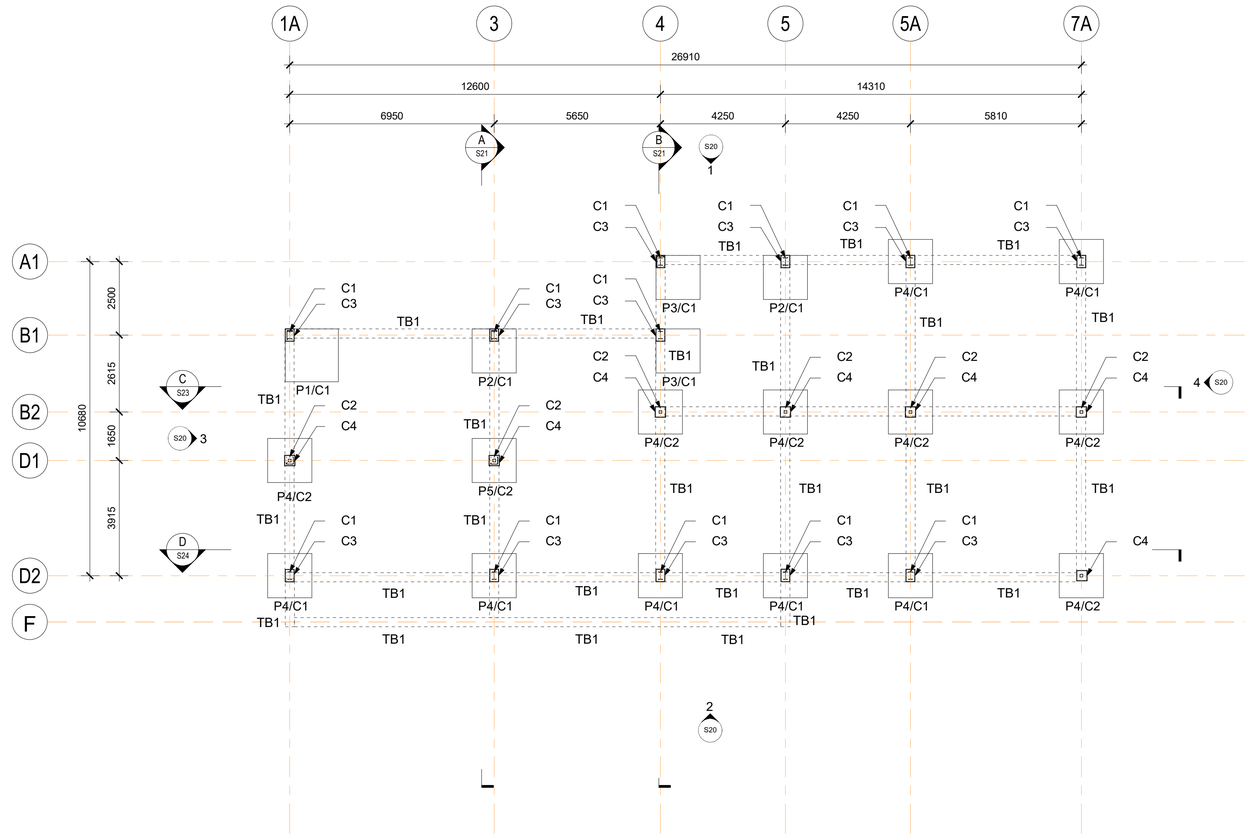
Drawing Title:
GENERAL NOTES (SHEET 4 OF 4)

Job No: 22507209

Drawing No: G04

Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A



1 S - FOUNDATION LAYOUT PLAN

500-03 Scale : 1 : 100

NOTES:

1. ALL UNITS ARE IN MILLIMETERS (UNO).
2. FOUNDATION TO BE ON UNDISTURBED NATURAL GROUND HAVING AN ULTIMATE BEARING STRENGTH OF 360 kPa (ALLOWABLE STRENGTH OF 120 kPa) SHOULD THE MATERIAL IMMEDIATELY UNDER THE FOUNDATION HAVE LESS STRENGTH THAN THIS, EXCAVATE A MINIMUM OF 200mm AND BACKFILL WITH GAP40 GRANULAR HARD-FILL COMPACTED TO A MINIMUM CBR OF 20. ALL FOUNDATION SUBGRADE AND BASE PREPARATION TO BE AS PER GEOTECHNICAL ENGINEER'S SPECIFICATION/RECOMMENDATION IN ANY INCONSISTENCY.
3. CONTRACTOR TO PROVIDE ALL ONSITE TESTING RESULT TO GEOTECHNICAL ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION OF FOUNDATION ANY FOREIGN MATERIAL TO BE TESTED AND APPROVED BY GEOTECHNICAL ENGINEER.
4. ALL UNSUITABLE EXCAVATED MATERIAL SHALL BE REMOVED FROM THE SITE. THIS MATERIAL SHALL NOT BE USED FOR LEVELING, FILLING, EXCESS EXCAVATIONS OR BACKFILLING BEHIND RETAINING WALLS UNLESS WRITTEN APPROVAL HAS BEEN OBTAINED FROM THE ENGINEER.
5. ALL BACKFILL MATERIAL SHALL BE FULLY COMPACTED TO 98% MAXIMUM DRY DENSITY.
6. MINIMUM CHARACTERISTIC COMPRESSIVE STRENGTH OF CONCRETE (F'C) SHALL BE 32 MPA AT 28 DAYS. AS PER B2 EXPOSURE CLASSIFICATIONS TO NZS 3101:2016 A1-A3 TABLE 3.6.
7. ALL THE FOUNDATION SETOUT DIMENSIONS AND LEVELS TO BE TAKEN FROM THE ARCHITECTURAL DRAWING.
8. ALL THE REINFORCEMENT COVER TO BE IN ACCORDANCE TO THE MENTIONED ALL TYPE REINFORCEMENT COVERS IN THE GENERAL NOTES OR PROVIDED IN THE TYPICAL DETAIL DRAWINGS.

SCHEDULES

NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
P1/C1	1800 x 1800 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P2/C1	1500 x 1500 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P3/C1	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P4/C1	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P4/C2	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P5/C2	1500 x 1500 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
TB1	300mm x 450mm DEEP TIE BEAM REINFORCED WITH 6/HD 16 MAIN BARS AND 1/HD10 TIES @ 200c/c MAX WITH 1/HD10 CONFINEMENT TIES @ 200c/c.

NOTE:
THE TIE BEAM SHOWN IN THE PAD FOUNDATION LAYOUT IS FOR PRESENBTATTION PURPOSES HOWEVER THE ACTUAL LOCATION OF THE TIE IS SAME LEVEL AS FLOOR AND ITS INTERGRATED WITH THE ON GROUND FLOOR SLAB.

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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Plot By:
Plot Date:

QUALITY ASSURANCE
_____ DIRECTOR
_____ DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

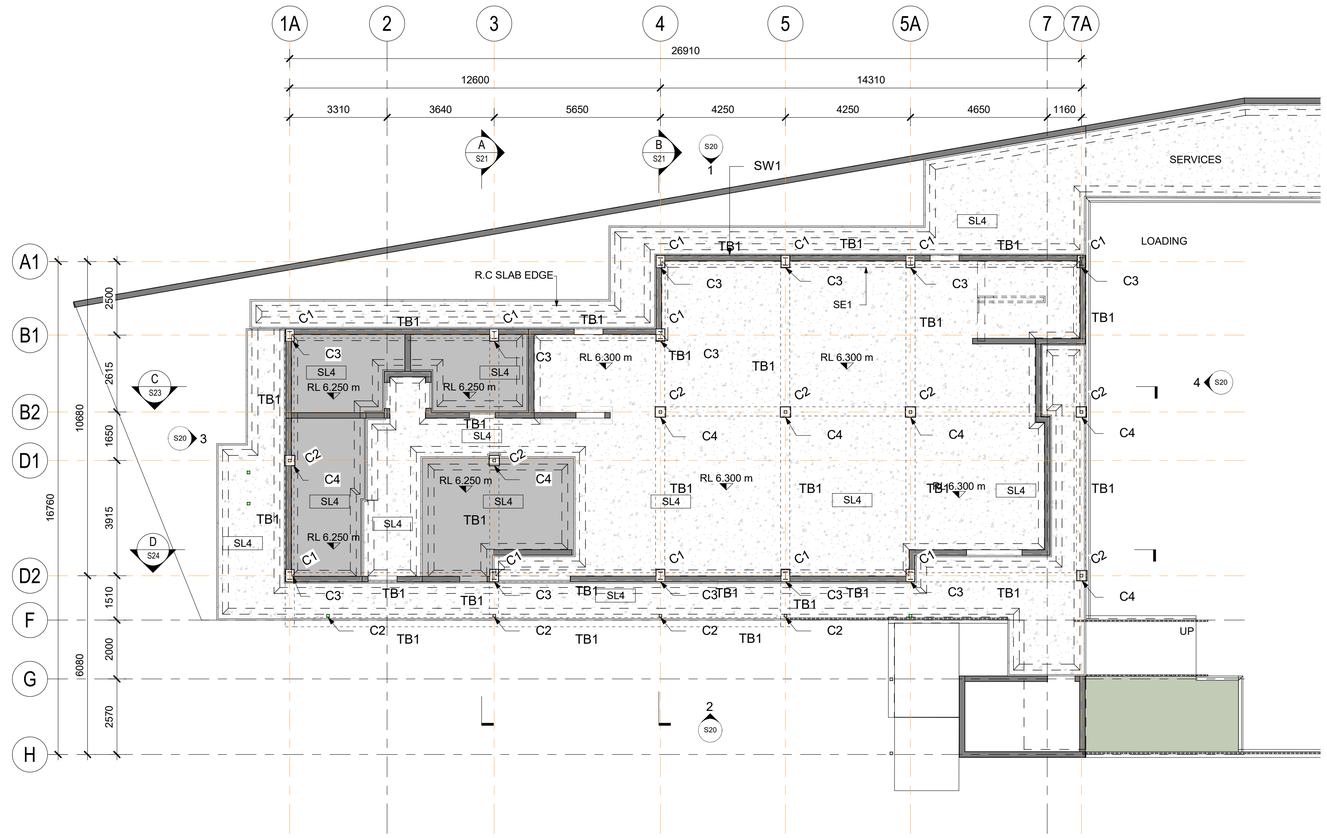
Drawing Title:
FOUNDATION LAYOUT PLAN

Job No: 22507209

Drawing No: S01

Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A



NOTES:

1. ALL UNITS ARE IN MILLIMETERS (UNO).
2. FOUNDATION TO BE ON UNDISTURBED NATURAL GROUND HAVING AN ULTIMATE BEARING STRENGTH OF 360 kPa (ALLOWABLE STRENGTH OF 120 kPa) SHOULD THE MATERIAL IMMEDIATELY UNDER THE FOUNDATION HAVE LESS STRENGTH THAN THIS, EXCAVATE A MINIMUM OF 200mm AND BACKFILL WITH GAP40 GRANULAR HARD-FILL COMPACTED TO A MINIMUM CBR OF 20. ALL FOUNDATION SUBGRADE AND BASE PREPARATION TO BE AS PER GEOTECHNICAL ENGINEER'S SPECIFICATION/RECOMMENDATION IN ANY INCONSISTENCY.
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5. ALL BACKFILL MATERIAL SHALL BE FULLY COMPACTED TO 98% MAXIMUM DRY DENSITY.
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7. ALL THE FOUNDATION SETOUT DIMENSIONS AND LEVELS TO BE TAKEN FROM THE ARCHITECTURAL DRAWING.
8. ALL THE REINFORCEMENT COVER TO BE IN ACCORDANCE TO THE MENTIONED ALL TYPE REINFORCEMENT COVERS IN THE GENERAL NOTES OR PROVIDED IN THE TYPICAL DETAIL DRAWINGS.

SCHEDULES

NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
SE1	R.C SLAB EDGE
SL4	125mm THCK. R.C SLAB WITH 665MESH 40CVR
SW1	150mm THK MASONRY WALL (FULLY GROUTED) WITH A 15mm THCK. MIN. PLASTER
TB1	300mm x 450mm DEEP TIE BEAM REINFORCED WITH 6/HD 16 MAIN BARS AND 1/HD10 TIES @ 200c/c MAX WITH 1/HD10 CONFINEMENT TIES @ 200c/c.

1 S - GROUND FLOOR LAYOUT PLAN

500-03 Scale : 1 : 100

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

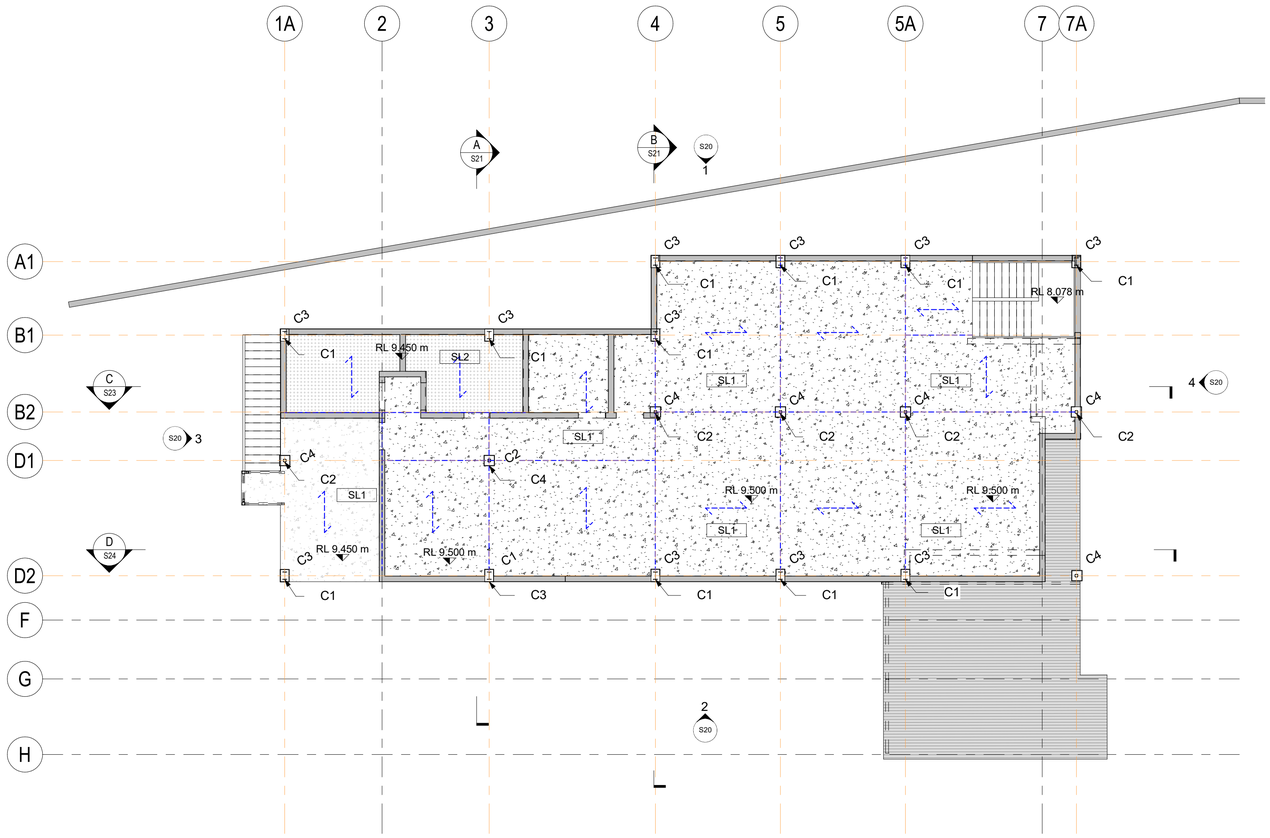
Drawing Title:
GROUND FLOOR LAYOUT PLAN

Job No: 22507209

Drawing No: S02

Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A



1 S - LEVEL 1 FLOOR LAYOUT PLAN

500-03 Scale : 1 : 100

NOTES:

1. ALL UNITS ARE IN MILLIMETERS (UNO).
2. FOUNDATION TO BE ON UNDISTURBED NATURAL GROUND HAVING AN ULTIMATE BEARING STRENGTH OF 300 kPa (ALLOWABLE STRENGTH OF 100 kPa) SHOULD THE MATERIAL IMMEDIATELY UNDER THE FOUNDATION HAVE LESS STRENGTH THAN THIS, EXCAVATE A MINIMUM OF 200mm AND BACKFILL WITH GAP40 GRANULAR HARD-FILL COMPACTED TO A MINIMUM CBR OF 20. ALL FOUNDATION SUBGRADE AND BASE PREPARATION TO BE AS PER GEOTECHNICAL ENGINEER'S SPECIFICATION/RECOMMENDATION IN ANY INCONSISTENCY.
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6. MINIMUM CHARACTERISTIC COMPRESSIVE STRENGTH OF CONCRETE (FC) SHALL BE 32 MPA AT 28 DAYS. AS PER B2 EXPOSURE CLASSIFICATIONS TO NZS 3101:2016 A1-A3 TABLE 3.6.
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8. ALL THE REINFORCEMENT COVER TO BE IN ACCORDANCE TO THE MENTIONED ALL TYPE REINFORCEMENT COVERS IN THE GENERAL NOTES OR PROVIDED IN THE TYPICAL DETAIL DRAWINGS.

SCHEDULES

NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
SL1	170mm THCK. R.C BONDEK SLAB
SL2	150mm THCK. R.C BONDEK SLAB

LEGEND:

- BONDEK SLAB DIRECTION

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REVISIONS	No	By	Date	Description	Ckd	Appr
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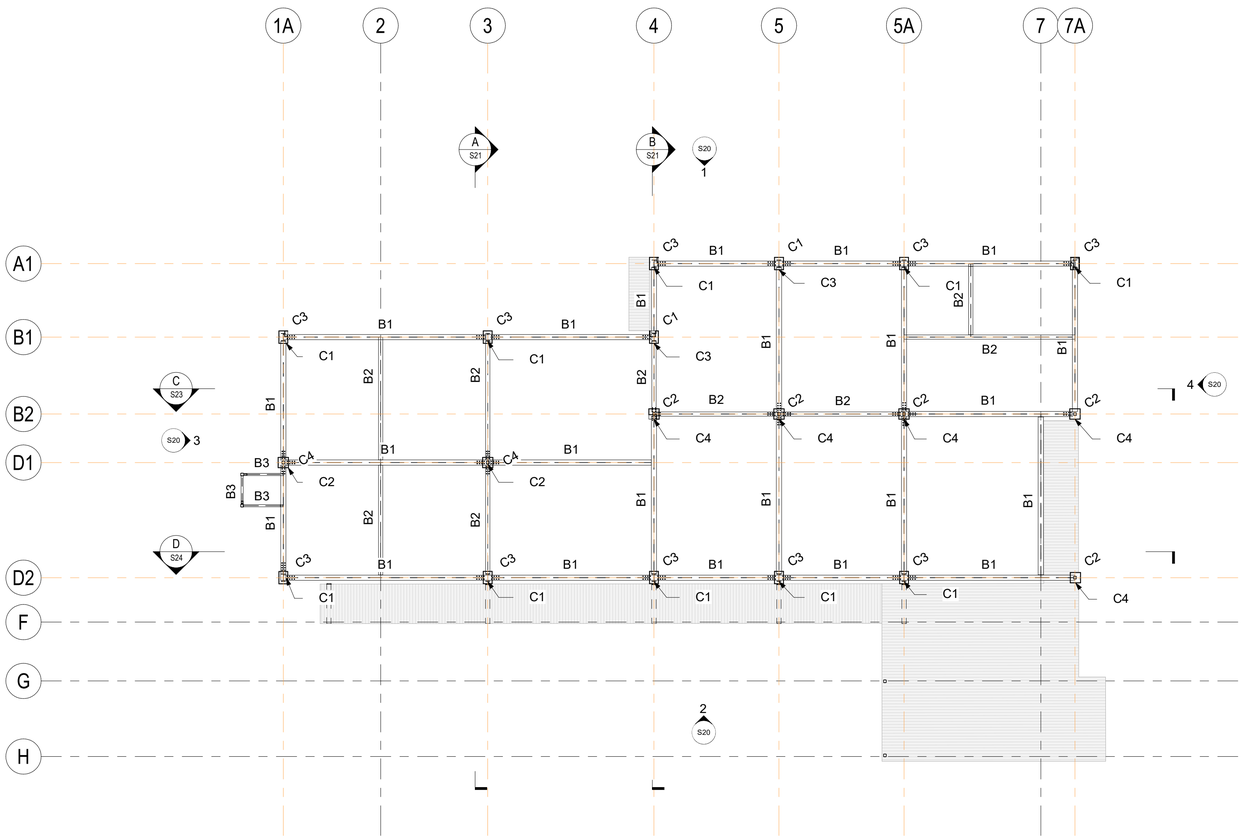
Drawing Title:
FIRST FLOOR SLAB LAYOUT PLAN

Job No: 22507209

Drawing No: S03

Scales:
A1 AS SHOWN
A3 HALF SHOWN

REVISION A



1 S - LEVEL 1 FLOOR FRAMING PLAN

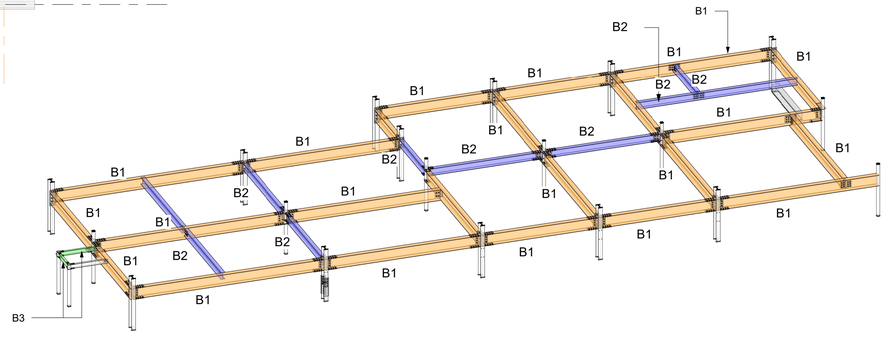
500-03 Scale : 1 : 100

NOTES:

1. ALL UNITS ARE IN MILLIMETERS (UNO).
2. FOUNDATION TO BE ON UNDISTURBED NATURAL GROUND HAVING AN ULTIMATE BEARING STRENGTH OF 300 kPa (ALLOWABLE STRENGTH OF 100 kPa) SHOULD THE MATERIAL IMMEDIATELY UNDER THE FOUNDATION HAVE LESS STRENGTH THAN THIS, EXCAVATE A MINIMUM OF 200mm AND BACKFILL WITH GAP40 GRANULAR HARD-FILL COMPACTED TO A MINIMUM CBR OF 20. ALL FOUNDATION SUBGRADE AND BASE PREPARATION TO BE AS PER GEOTECHNICAL ENGINEER'S SPECIFICATION/RECOMMENDATION IN ANY INCONSISTENCY.
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SCHEDULES

NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM
B3	150UB 15.0 STEEL FLOOR BEAM
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT



2 S04 - TYPICAL LEVEL FLOOR FRAMING VIEW

Scale :

FOR TENDER

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RD SOLOMON ISLAND

Drawing Title:
FIRST FLOOR FRAMING LAYOUT PLAN

Job No: 22507209

Drawing No: S04

Scales:
A1 AS SHOWN
A3 HALF SHOWN

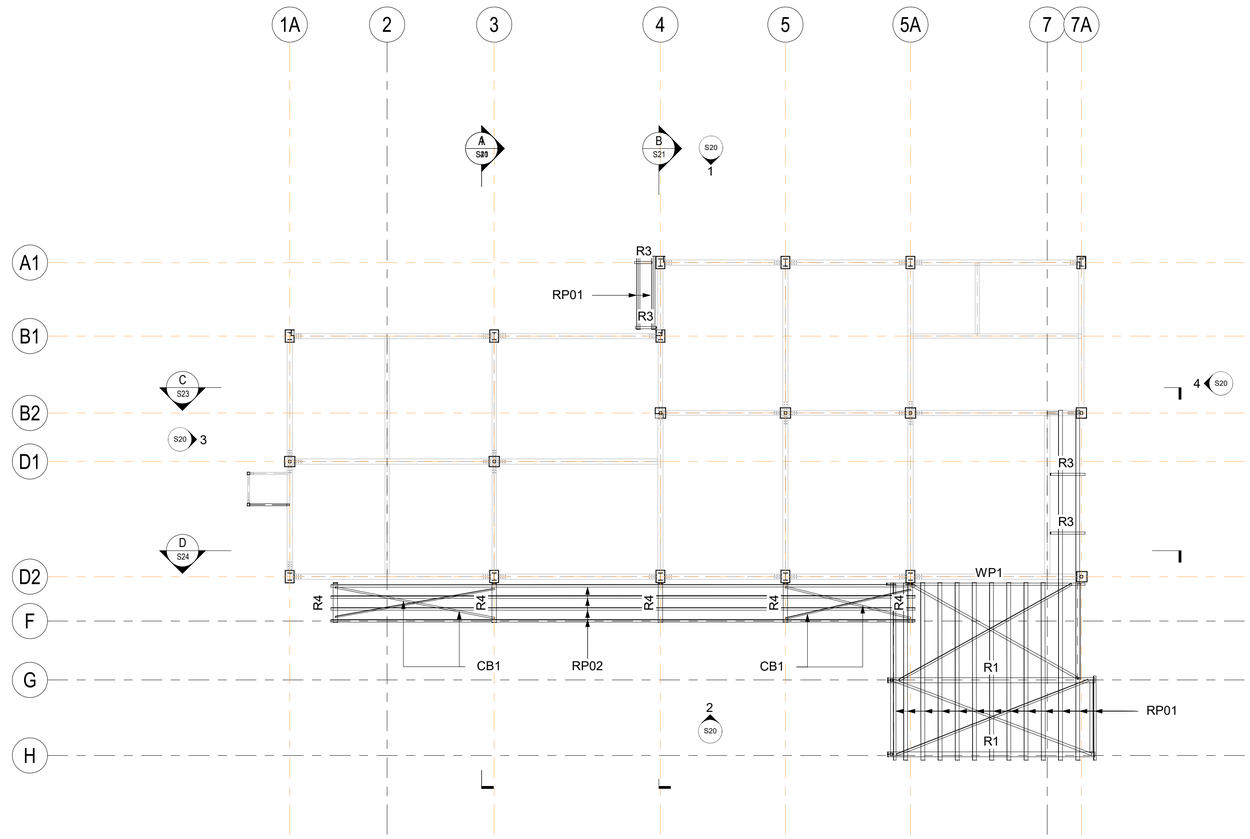
REVISION A

NOTES:

1. ALL UNITS ARE IN MILLIMETERS (UNO).
2. FOUNDATION TO BE ON UNDISTURBED NATURAL GROUND HAVING AN ULTIMATE BEARING STRENGTH OF 300 kPa (ALLOWABLE STRENGTH OF 100 kPa) SHOULD THE MATERIAL IMMEDIATELY UNDER THE FOUNDATION HAVE LESS STRENGTH THAN THIS, EXCAVATE A MINIMUM OF 200mm AND BACKFILL WITH GAP40 GRANULAR HARD-FILL COMPACTED TO A MINIMUM CBR OF 20. ALL FOUNDATION SUBGRADE AND BASE PREPARATION TO BE AS PER GEOTECHNICAL ENGINEER'S SPECIFICATION/RECOMMENDATION IN ANY INCONSISTENCY.
3. CONTRACTOR TO PROVIDE ALL ONSITE TESTING RESULT TO GEOTECHNICAL ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION OF FOUNDATION ANY FOREIGN MATERIAL TO BE TESTED AND APPROVED BY GEOTECHNICAL ENGINEER.
4. ALL UNSUITABLE EXCAVATED MATERIAL SHALL BE REMOVED FROM THE SITE. THIS MATERIAL SHALL NOT BE USED FOR LEVELING, FILLING, EXCESS EXCAVATIONS OR BACKFILLING BEHIND RETAINING WALLS UNLESS WRITTEN APPROVAL HAS BEEN OBTAINED FROM THE ENGINEER.
5. ALL BACKFILL MATERIAL SHALL BE FULLY COMPACTED TO 98% MAXIMUM DRY DENSITY.
6. MINIMUM CHARACTERISTIC COMPRESSIVE STRENGTH OF CONCRETE (FC) SHALL BE 32 MPA AT 28 DAYS. AS PER B2 EXPOSURE CLASSIFICATIONS TO NZS 3101:2016 A1-A3 TABLE 3.6.
7. ALL THE FOUNDATION SETOUT DIMENSIONS AND LEVELS TO BE TAKEN FROM THE ARCHITECTURAL DRAWING.
8. ALL THE REINFORCEMENT COVER TO BE IN ACCORDANCE TO THE MENTIONED ALL TYPE REINFORCEMENT COVERS IN THE GENERAL NOTES OR PROVIDED IN THE TYPICAL DETAIL DRAWINGS.

SCHEDULES

NUMBER	DESCRIPTION
CB1	75EA 9.0 STEEL ROOF BRACING
R1	360UB 37.3 STEEL RAFTER
R3	150UB 14.0 STEEL RAFTER
R4	250x150x6.0 RHS STEEL RAFTER
RP01	Z15075 STEEL PURLINS @ 600 - 900CRS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO STEEL RAFTERS AND FIXED TO THE PURLINS WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION
RP02	C200 STEEL END PURLINS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO THE RAFTER AND FIXED TO THE PURLIN WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION
WP1	150PFC STEEL WALL PLATE FIXED TO EXTERNAL MASONRY WALL USING M16 8.8/S CHEMSET BOLTS WITH WASHERS @ 1000CRS



1 S - LOWER ROOF FRAMING PLAN

500-03 Scale : 1 : 100

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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Client

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NRW MACALLAN (FIJI) LTD
CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE	
Checked: RC	Date: 08/14/25	CAD File:	
Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	

QUALITY ASSURANCE

_____ DIRECTOR _____ DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

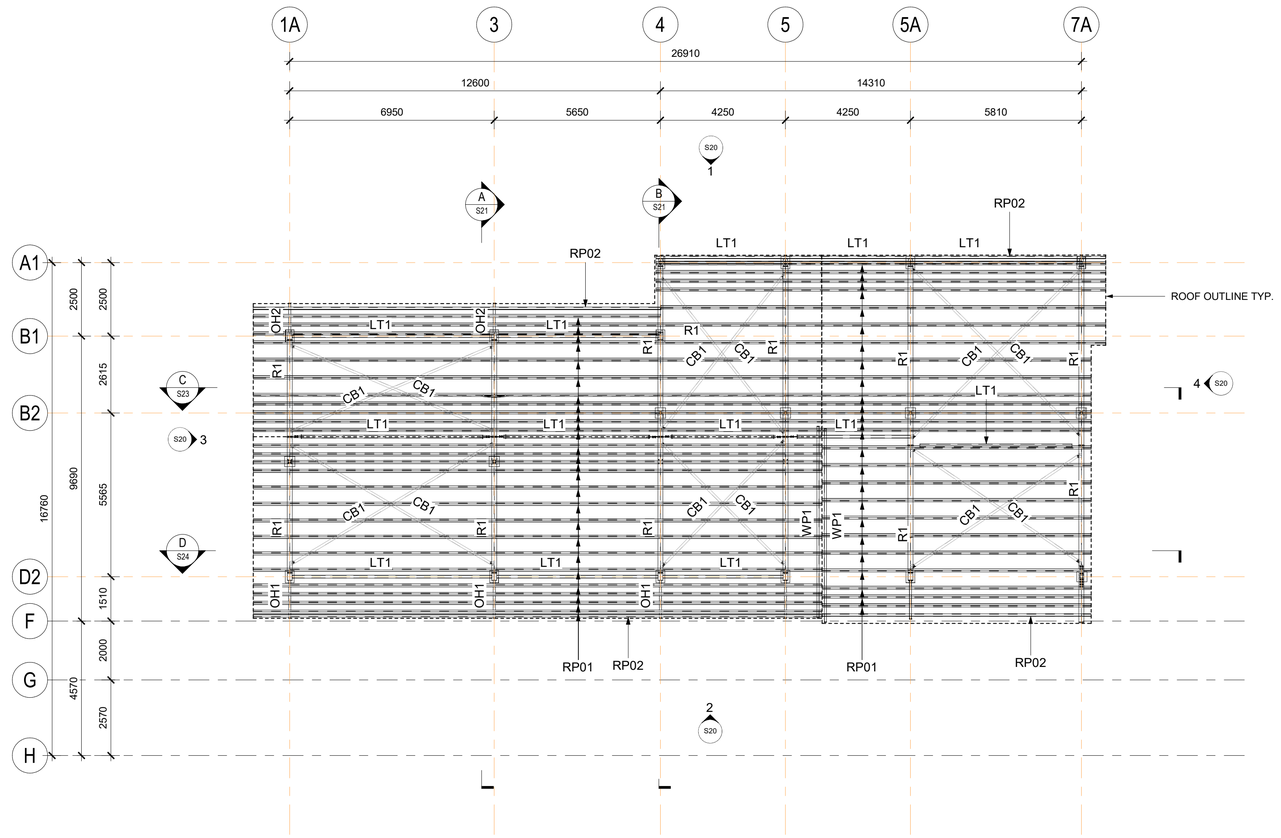
Drawing Title:
LOWER ROOF FRAMING LAYOUT PLAN

Job No: 22507209

Drawing No: S05

Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A



1 S - UPPER ROOF FRAMING PLAN

S20 Scale : 1 : 100

NOTES:

1. ALL UNITS ARE IN MILLIMETERS (UNO).
2. FOUNDATION TO BE ON UNDISTURBED NATURAL GROUND HAVING AN ULTIMATE BEARING STRENGTH OF 300 kPa (ALLOWABLE STRENGTH OF 100 kPa) SHOULD THE MATERIAL IMMEDIATELY UNDER THE FOUNDATION HAVE LESS STRENGTH THAN THIS, EXCAVATE A MINIMUM OF 200mm AND BACKFILL WITH GAP40 GRANULAR HARD-FILL COMPACTED TO A MINIMUM CBR OF 20. ALL FOUNDATION SUBGRADE AND BASE PREPARATION TO BE AS PER GEOTECHNICAL ENGINEER'S SPECIFICATION/RECOMMENDATION IN ANY INCONSISTENCY.
3. CONTRACTOR TO PROVIDE ALL ONSITE TESTING RESULT TO GEOTECHNICAL ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION OF FOUNDATION ANY FOREIGN MATERIAL TO BE TESTED AND APPROVED BY GEOTECHNICAL ENGINEER.
4. ALL UNSUITABLE EXCAVATED MATERIAL SHALL BE REMOVED FROM THE SITE. THIS MATERIAL SHALL NOT BE USED FOR LEVELING, FILLING, EXCESS EXCAVATIONS OR BACKFILLING BEHIND RETAINING WALLS UNLESS WRITTEN APPROVAL HAS BEEN OBTAINED FROM THE ENGINEER.
5. ALL BACKFILL MATERIAL SHALL BE FULLY COMPACTED TO 98% MAXIMUM DRY DENSITY.
6. MINIMUM CHARACTERISTIC COMPRESSIVE STRENGTH OF CONCRETE (FC) SHALL BE 32 MPA AT 28 DAYS. AS PER B2 EXPOSURE CLASSIFICATIONS TO NZS 3101:2016 A1-A3 TABLE 3.6.
7. ALL THE FOUNDATION SETOUT DIMENSIONS AND LEVELS TO BE TAKEN FROM THE ARCHITECTURAL DRAWING.
8. ALL THE REINFORCEMENT COVER TO BE IN ACCORDANCE TO THE MENTIONED ALL TYPE REINFORCEMENT COVERS IN THE GENERAL NOTES OR PROVIDED IN THE TYPICAL DETAIL DRAWINGS.

SCHEDULES

NUMBER	DESCRIPTION
CB1	75EA 9.0 STEEL ROOF BRACING
LT1	100SHS 9.0 STEEL ROOF LATERAL TIE
OH1	150PFC STEEL OUTRIGGER RAFTER 1300mm OVERHANG
OH2	150PFC STEEL OUTRIGGER RAFTER 1000mm OVERHANG
R1	360UB 37.3 STEEL RAFTER
RP01	Z15075 STEEL PURLINS @ 600 - 900CRS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO STEEL RAFTERS AND FIXED TO THE PURLINS WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION
RP02	C200 STEEL END PURLINS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO THE RAFTER AND FIXED TO THE PURLIN WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION
WP1	150PFC STEEL WALL PLATE FIXED TO EXTERNAL MASONRY WALL USING M16 8.8/S CHEMSET BOLTS WITH WASHERS @ 1000CRS

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE	
Checked: RC	Date: 08/14/25	CAD File:	
Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	

QUALITY ASSURANCE

_____ DIRECTOR _____ DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

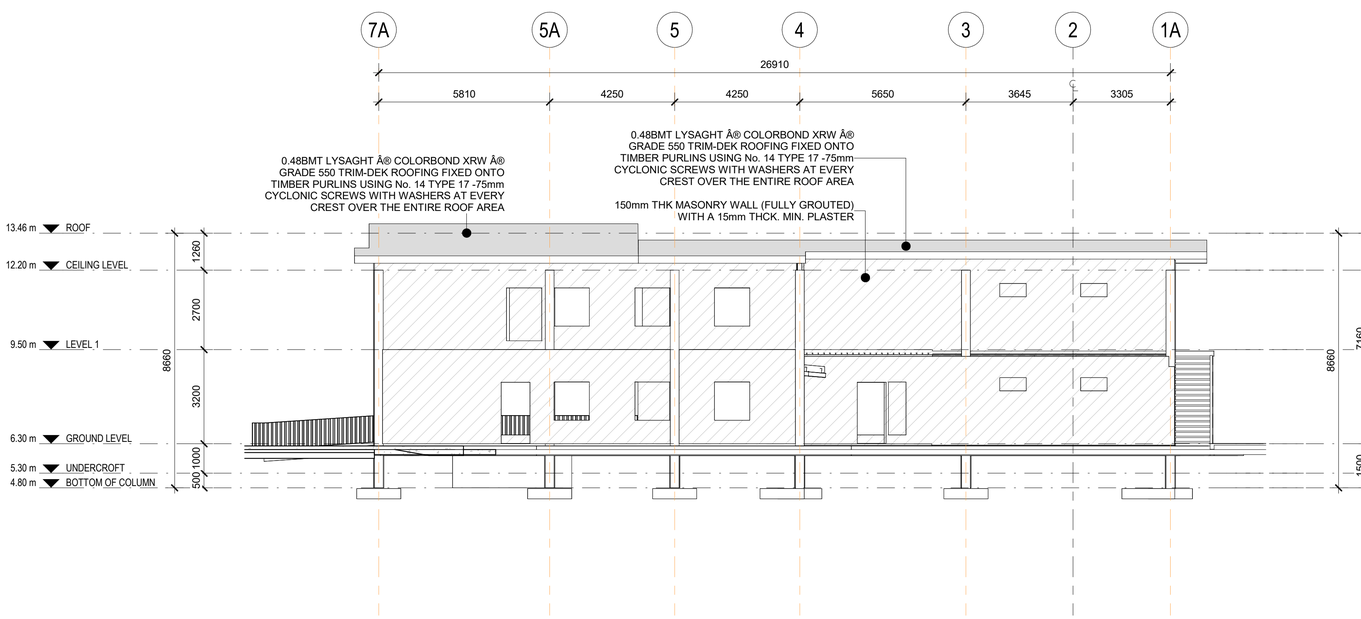
Drawing Title:
UPPER ROOF FRAMING LAYOUT PLAN

Job No: 22507209

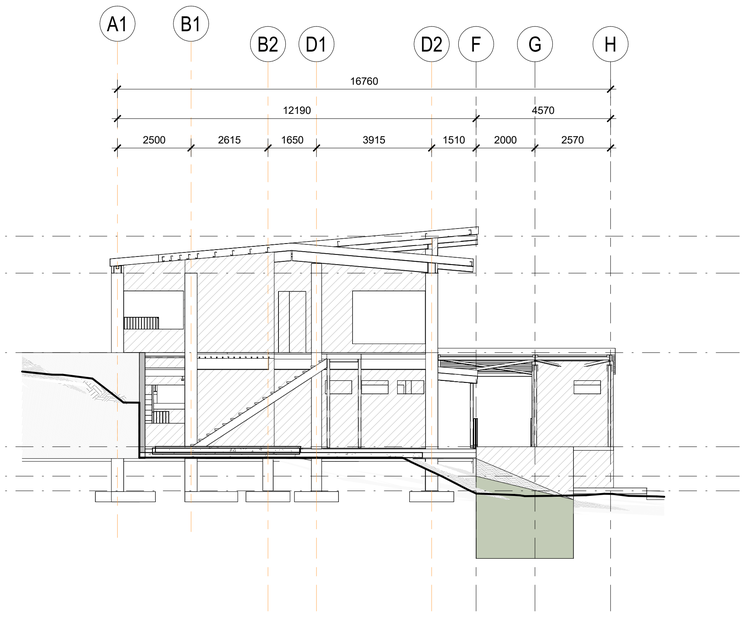
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Scales: A1 AS SHOWN; A3 HALF SHOWN

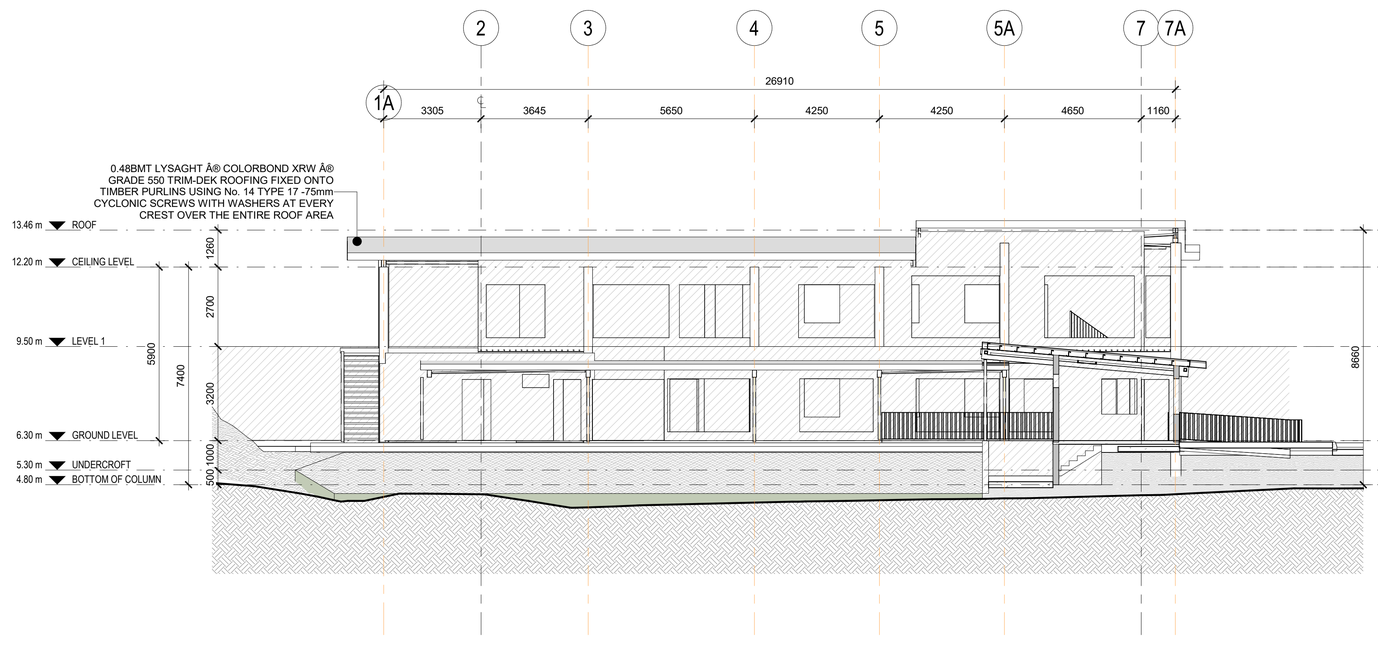
REVISION A



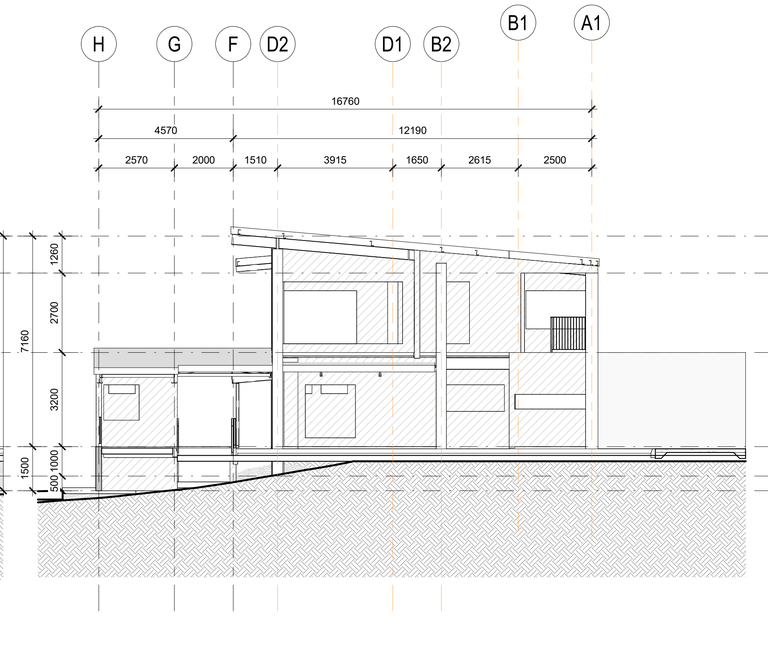
1 **NORTH**
S01 Scale : 1 : 100



3 **EAST**
S01 Scale : 1 : 100



2 **SOUTH**
S01 Scale : 1 : 100



4 **WEST**
S01 Scale : 1 : 100

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

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Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	

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DIRECTOR DATE

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RD SOLOMON ISLAND

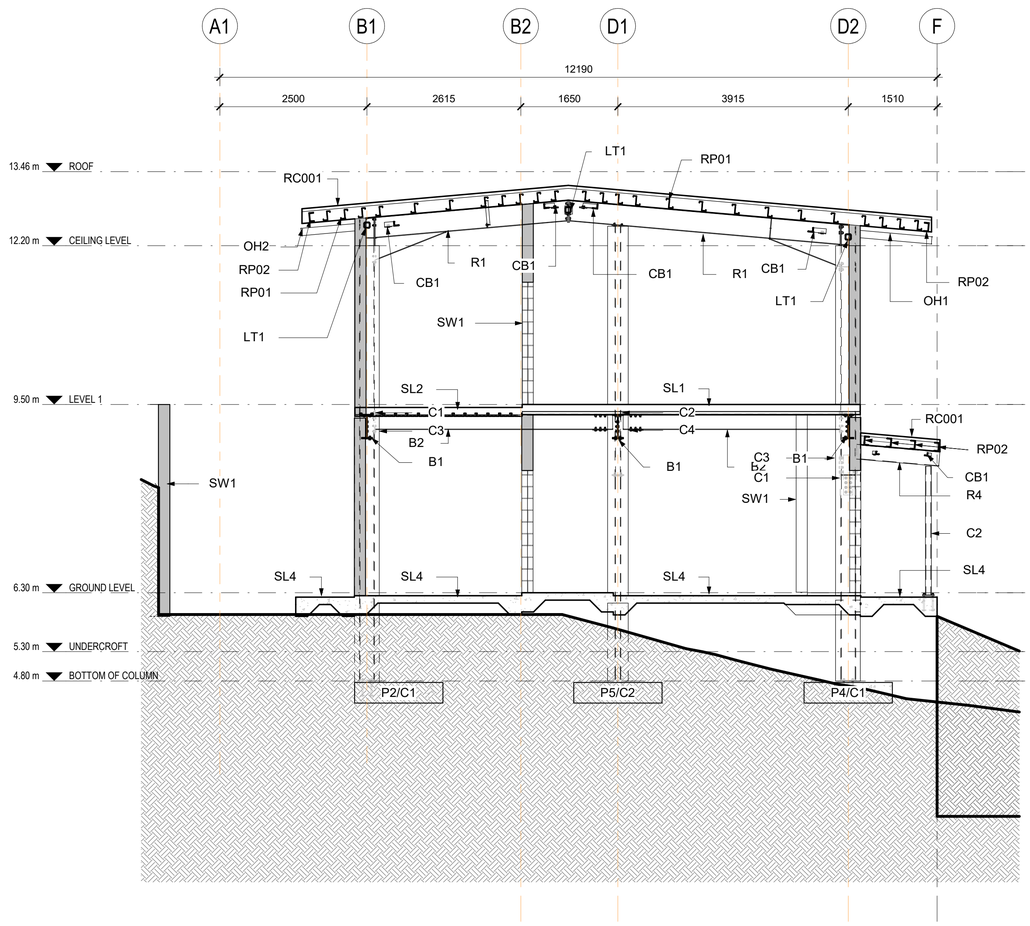
Drawing Title:
ELEVATIONS (SHEET 1 OF 2)

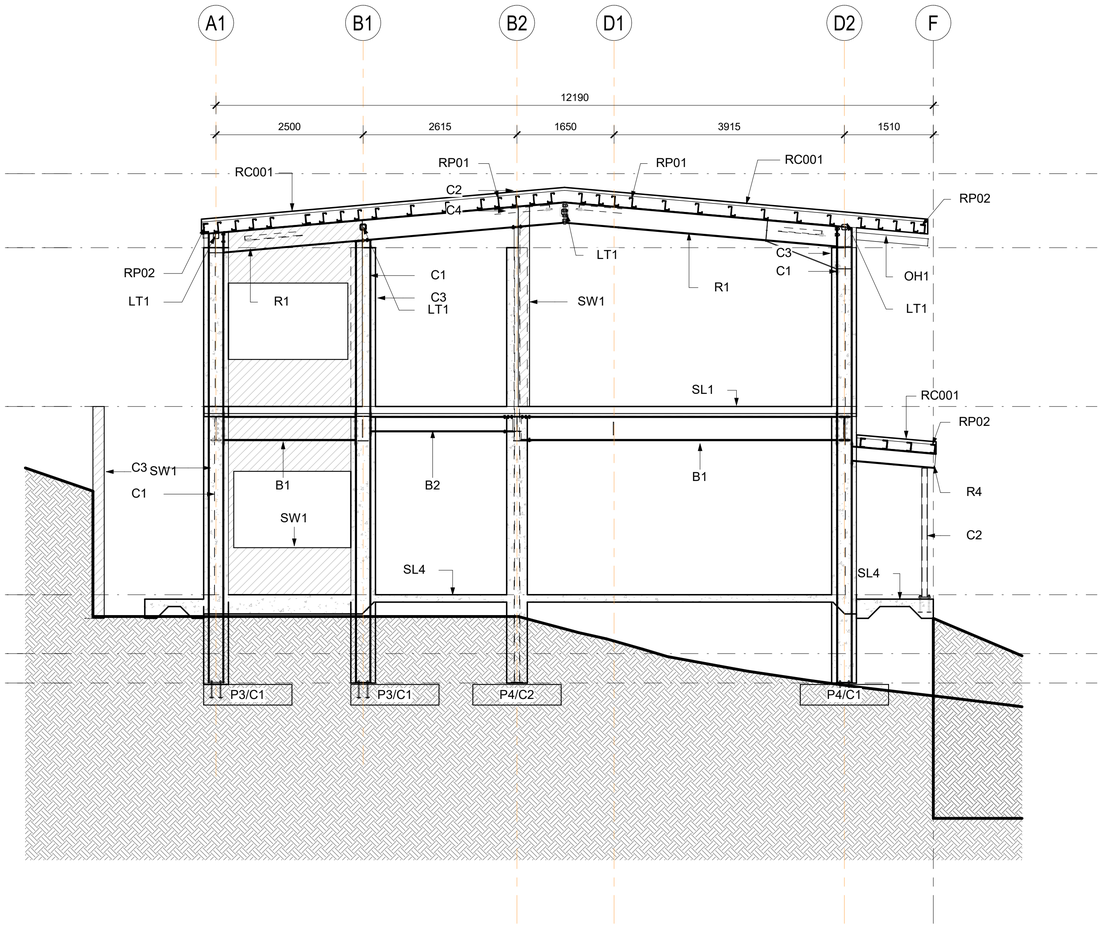
Job No: 22507209

Drawing No: S20

Scales:
A1 AS SHOWN
A3 HALF SHOWN

REVISION A





A SECTION 1

S01 Scale : 1 : 50

B SECTION 2

S01 Scale : 1 : 50

SCHEDULES

NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
CB1	75EA 9.0 STEEL ROOF BRACING
LT1	100SHS 9.0 STEEL ROOF LATERAL TIE
OH1	150PFC STEEL OUTRIGGER RAFTER 1300mm OVERHANG
OH2	150PFC STEEL OUTRIGGER RAFTER 1000mm OVERHANG
P2/C1	1500 x 1500 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P3/C1	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P4/C1	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND

SCHEDULES

NUMBER	DESCRIPTION
P4/C2	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P5/C2	1500 x 1500 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
R1	360UB 37.3 STEEL RAFTER
R4	250x150x6.0 RHS STEEL RAFTER
RC001	0.48BMT LYSAGHT ÃŠ COLORBOND XRW ÃŠ GRADE 550 TRIM-DEK ROOFING FIXED ONTO TIMBER PURLINS USING No. 14 TYPE 17 -75mm CYCLONIC SCREWS WITH WASHERS AT EVERY CREST OVER THE ENTIRE ROOF AREA
RP01	Z15075 STEEL PURLINS @ 600 - 900CRS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO STEEL RAFTERS AND FIXED TO THE PURLINS WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION
RP02	C200 STEEL END PURLINS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO THE RAFTER AND FIXED TO THE PURLIN WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION
SL1	170mm THCK. R.C BONDEK SLAB
SL2	150mm THCK. R.C BONDEK SLAB
SL4	125mm THCK. R.C SLAB WITH 665MESH 40CVR
SW1	150mm THK MASONRY WALL (FULLY GROUTED) WITH A 15mm THCK. MIN. PLASTER

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter:	EA	Date:	08/14/25
Checked:	RC	Date:	08/14/25
Designed:	RC	Date:	----
Approved:	NK	Date:	----

AUTOCAD REFERENCE
CAD File:
Plot By:
Plot Date:

QUALITY ASSURANCE
DIRECTOR DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

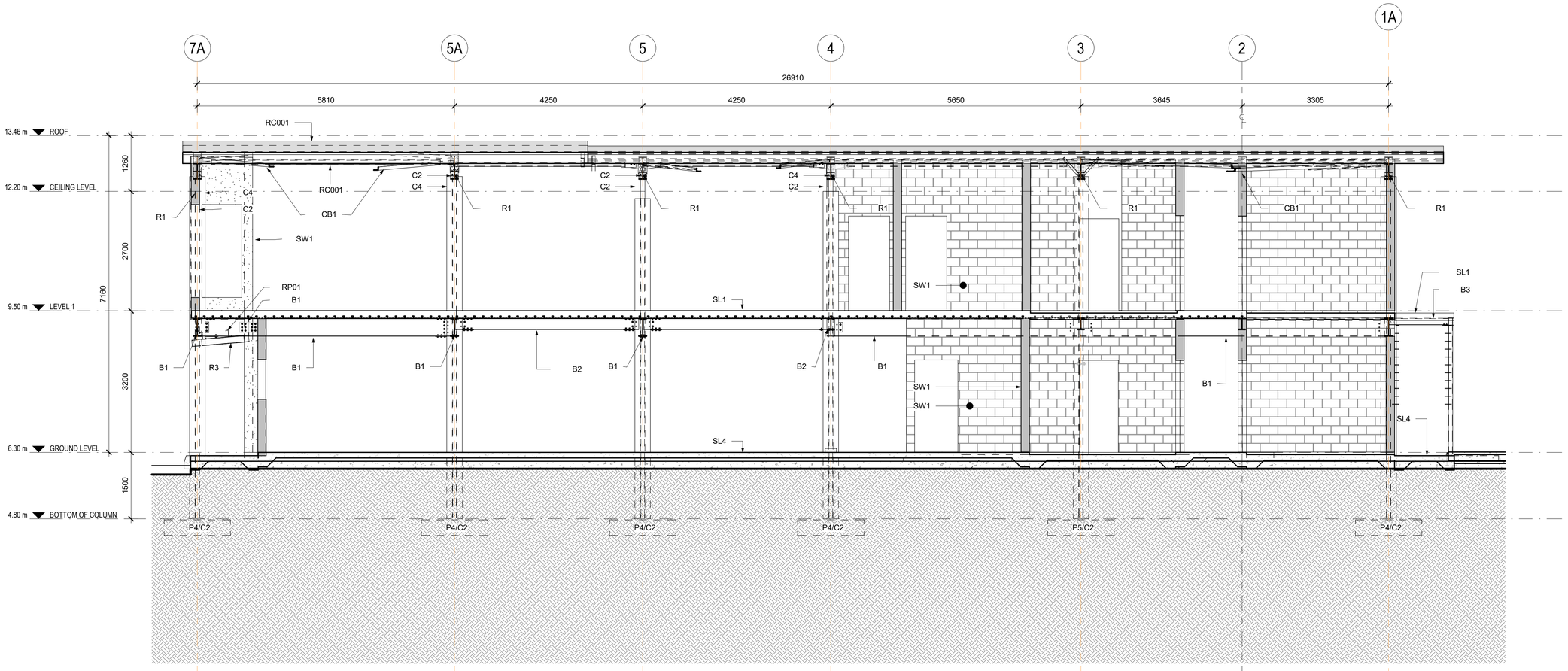
Drawing Title:
SECTION (SHEET 1 OF 3)

Job No: 22507209

Drawing No: S21

Scales:
A1 AS SHOWN
A3 HALF SHOWN

REVISION A



C SECTION 3

S01 Scale : 1 : 50

SCHEDULES	
NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM
B3	150UB 15.0 STEEL FLOOR BEAM
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
CB1	75EA 9.0 STEEL ROOF BRACING
P4/C2	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P5/C2	1500 x 1500 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
R1	360UB 37.3 STEEL RAFTER
R3	150UB 14.0 STEEL RAFTER
RC001	0.48BMT LYSAGHT Â® COLORBOND XRW Â® GRADE 550 TRIM-DEK ROOFING FIXED ONTO TIMBER PURLINS USING No. 14 TYPE 17 -75mm CYCLONIC SCREWS WITH WASHERS AT EVERY CREST OVER THE ENTIRE ROOF AREA
RP01	Z15075 STEEL PURLINS @ 600 - 900CRS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO STEEL RAFTERS AND FIXED TO THE PURLINS WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION
SL1	170mm THCK. R.C BONDEK SLAB
SL4	125mm THCK. R.C SLAB WITH 665MESH 40CVR
SW1	150mm THK MASONRY WALL (FULLY GROUTED) WITH A 15mm THCK. MIN. PLASTER

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE	
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QUALITY ASSURANCE

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Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

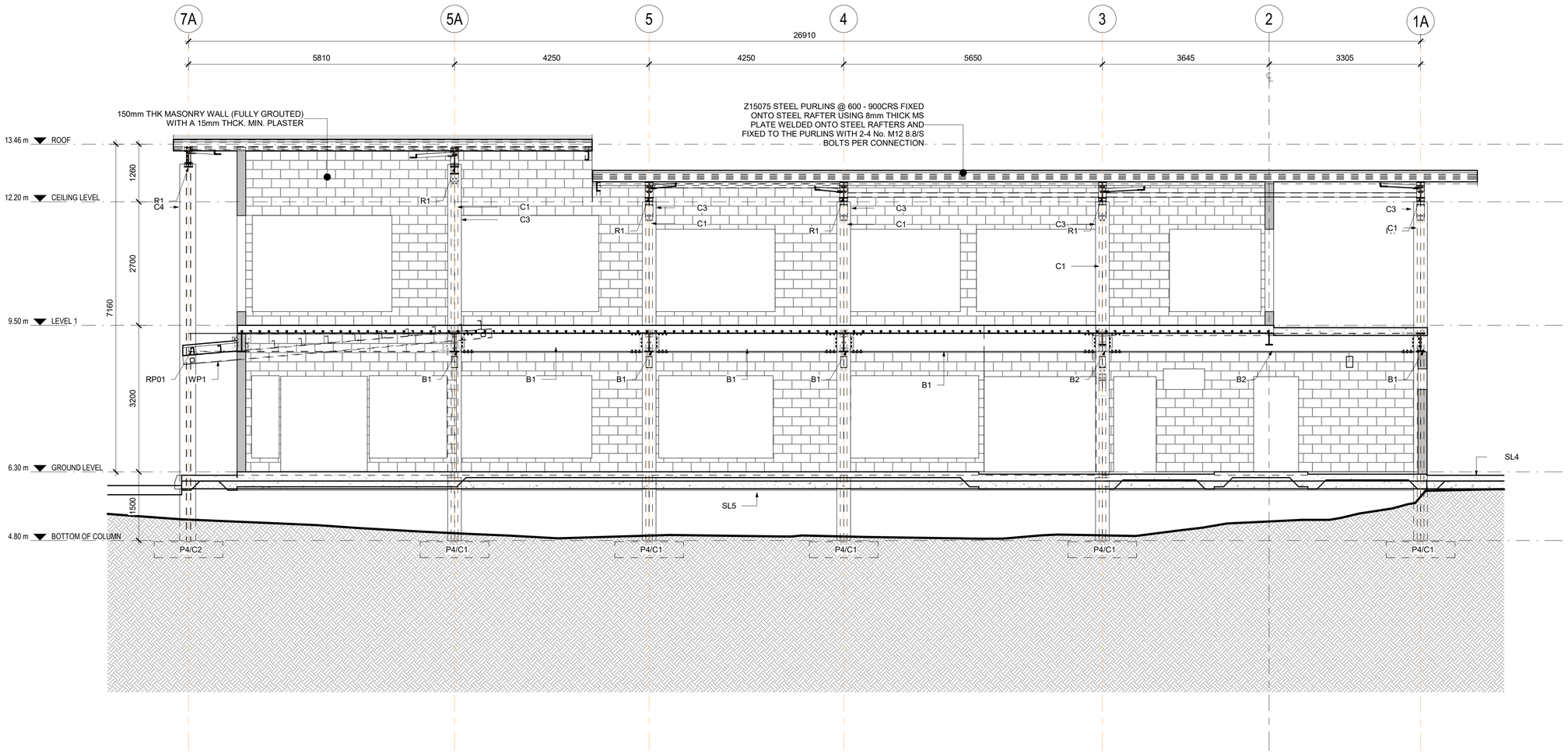
Job No: 22507209

Drawing No: S23

Drawing Title:
SECTION (SHEET 2 OF 3)

Scales:
A1 AS SHOWN
A3 HALF SHOWN

REVISION A



D SECTION 4

S01 Scale : 1 : 50

SCHEDULES

NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
P4/C1	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
P4/C2	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
R1	360UB 37.3 STEEL RAFTER
RP01	Z15075 STEEL PURLINS @ 600 - 900CRS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO STEEL RAFTERS AND FIXED TO THE PURLINS WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION
SL4	125mm THCK. R.C SLAB WITH 665MESH 40CVR
SL5	170mm THCK. R.C SLAB WITH HD12 @ 200CRS BOTHWAYS
SW1	150mm THK MASONRY WALL (FULLY GROUTED) WITH A 15mm THCK. MIN. PLASTER
WP1	150PFC STEEL WALL PLATE FIXED TO EXTERNAL MASONRY WALL USING M16 8.8/S CHEMSET BOLTS WITH WASHERS @ 1000CRS

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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Designed:	RC	Date:	----
Approved:	NK	Date:	----

AUTOCAD REFERENCE
CAD File:
Plot By:
Plot Date:

QUALITY ASSURANCE
DIRECTOR DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

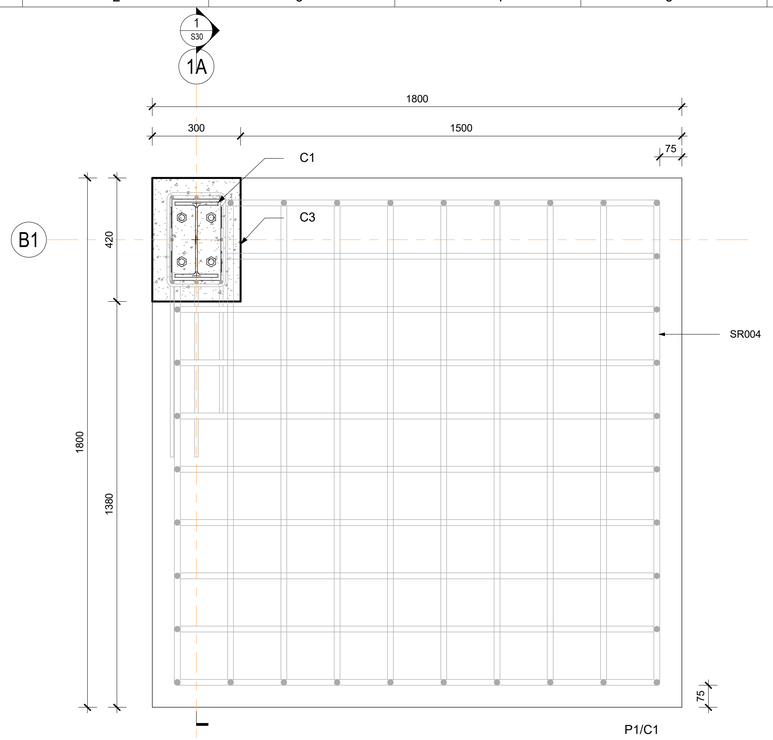
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SECTION (SHEET 3 OF 3)

Job No: 22507209

Drawing No: S24

Scales:
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A3 HALF SHOWN

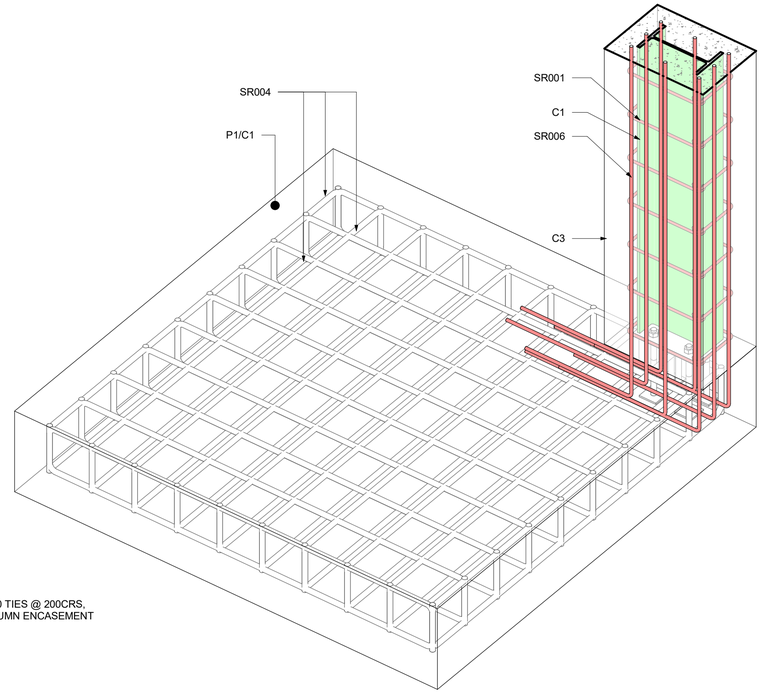
REVISION A



3 S30 - P1/C1 PLAN

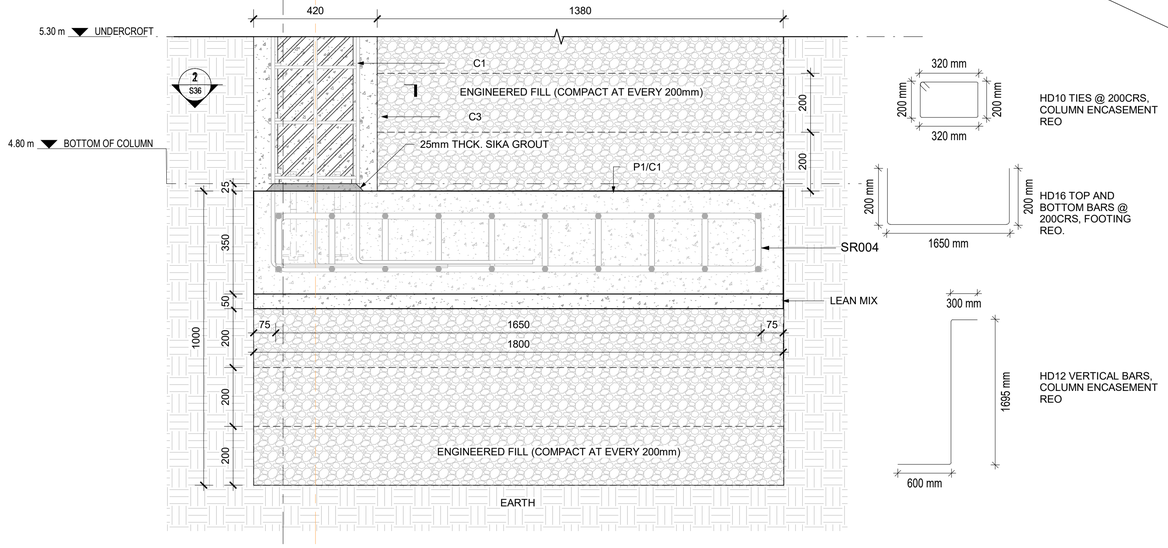
500-03 Scale : 1 : 10

SCHEDULES	
NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
P1/C1	1800 x 1800 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
SR001	HD10 TIES @ 200CRS
SR004	HD20 TOP AND BOTTOM BARS BOTHWAYS @ 150CRS
SR006	HD12 VERTICAL BARS, COLUMN ENCASEMENT



5 S30 - P1/C1 ISOMETRIC VIEW

Scale :



1 S30 - TYPICAL P1/C1 DETAIL

S30 Scale : 1 : 10

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REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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DIRECTOR DATE

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RD SOLOMON ISLAND

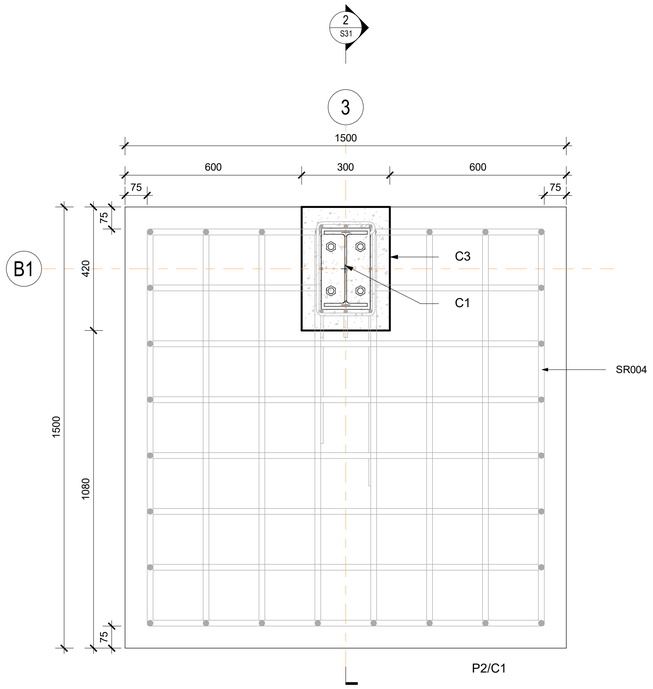
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TYPICAL P1/C1 FOOTING DETAILS

Job No: 22507209

Drawing No: S30

Scales: A1 AS SHOWN; A3 HALF SHOWN

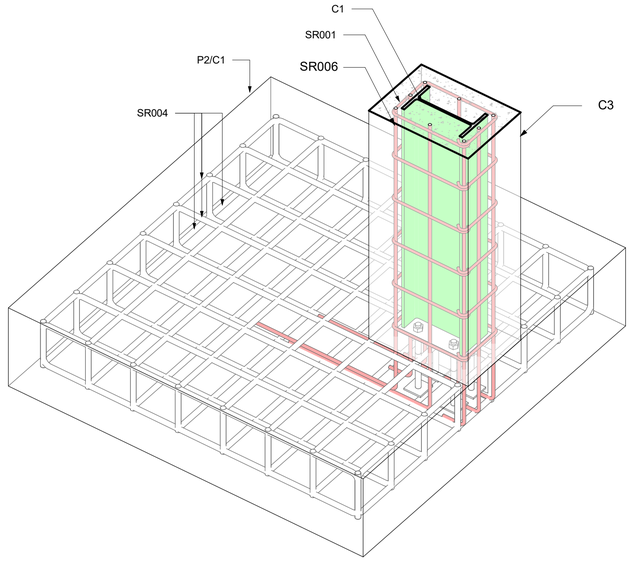
REVISION A



1 S31 - P2/C1 PLAN

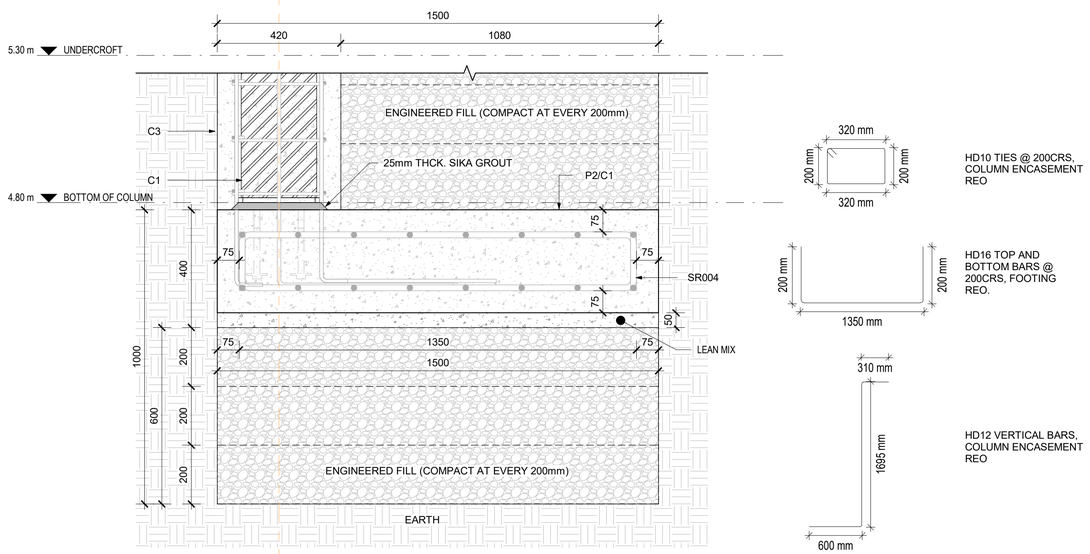
500-03 Scale : 1 : 10

SCHEDULES	
NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
P2/C1	1500 x 1500 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
SR001	HD10 TIES @ 200CRS
SR004	HD20 TOP AND BOTTOM BARS BOTHWAYS @ 150CRS
SR006	HD12 VERTICAL BARS, COLUMN ENCASEMENT



3 S31 - P2/C1 ISOMETRIC VIEW

Scale :



2 S31 - TYPICAL P2/C1 DETAIL

S31 Scale : 1 : 10

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE	
Checked: RC	Date: 08/14/25	CAD File:	
Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	
		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Drawing Title:
TYPICAL P2/C1 FOOTING DETAILS

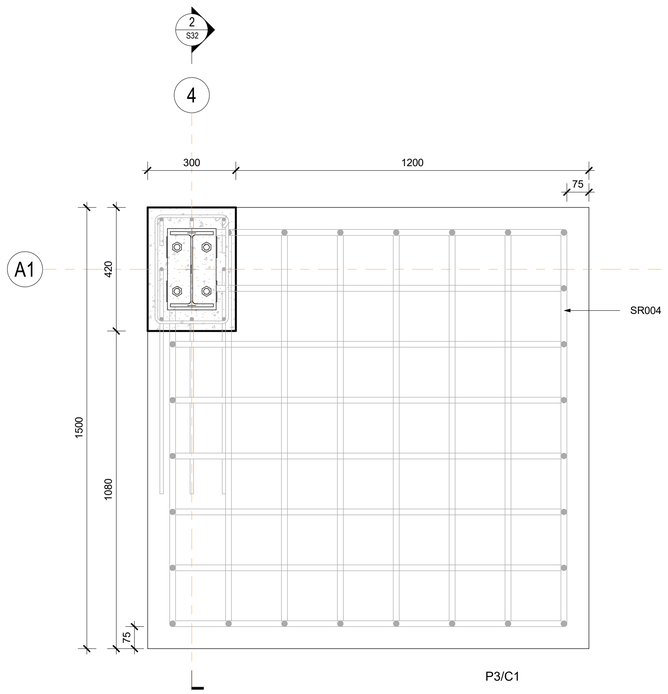
Job No: 22507209

Drawing No: S31

Scales: A1 AS SHOWN; A3 HALF SHOWN

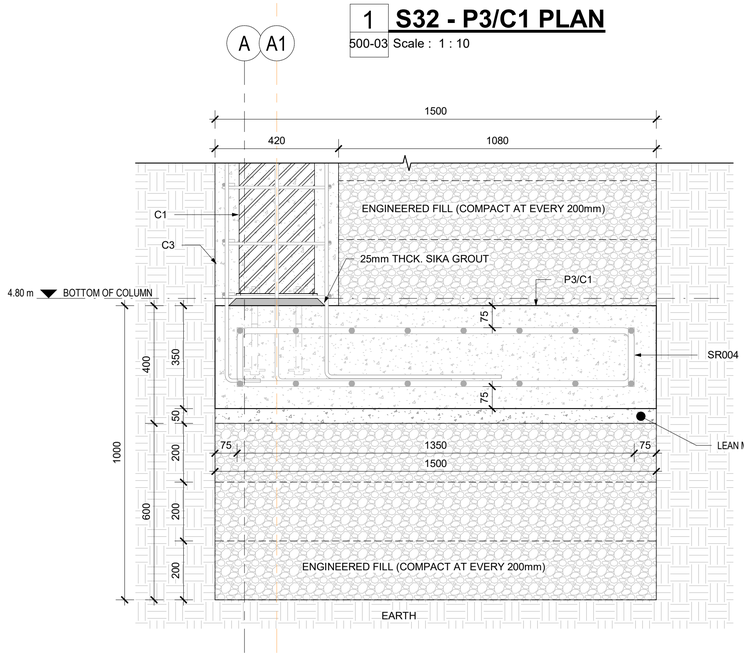
REVISION A

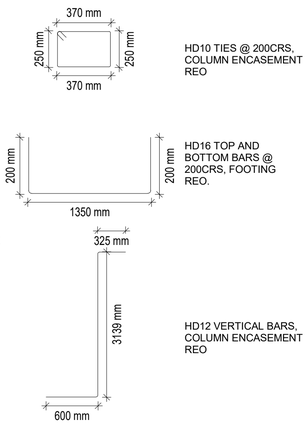
SCHEDULES	
NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
P3/C1	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
SR001	HD10 TIES @ 200CRS
SR004	HD20 TOP AND BOTTOM BARS BOTHWAYS @ 150CRS
SR006	HD12 VERTICAL BARS, COLUMN ENCASEMENT



1 S32 - P3/C1 PLAN

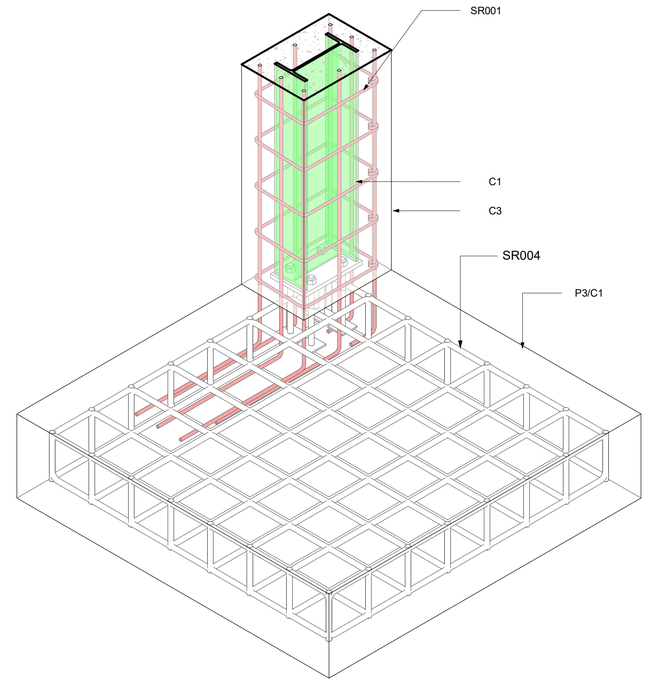
500-03 Scale : 1 : 10





2 S32 - TYPICAL P3/C1 DETAIL

S32 Scale : 1 : 10



3 S32 - P3/C1 ISOMETRIC VIEW

Scale :

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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P.O. BOX 1208, SUVA
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EMAIL: info@nrwmacallan.com.fj

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		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Job No: 22507209

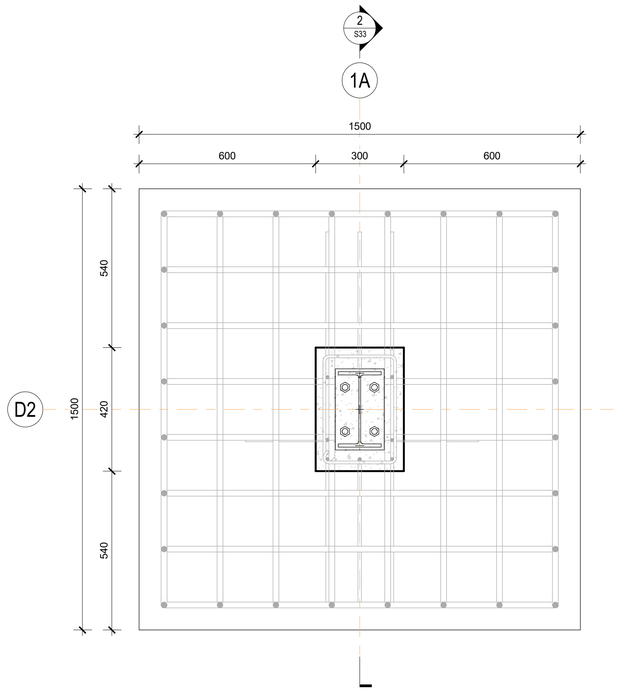
Drawing No: S32

Drawing Title:
TYPICAL P3/C1 FOOTING DETAILS

Scales:
A1 AS SHOWN
A3 HALF SHOWN

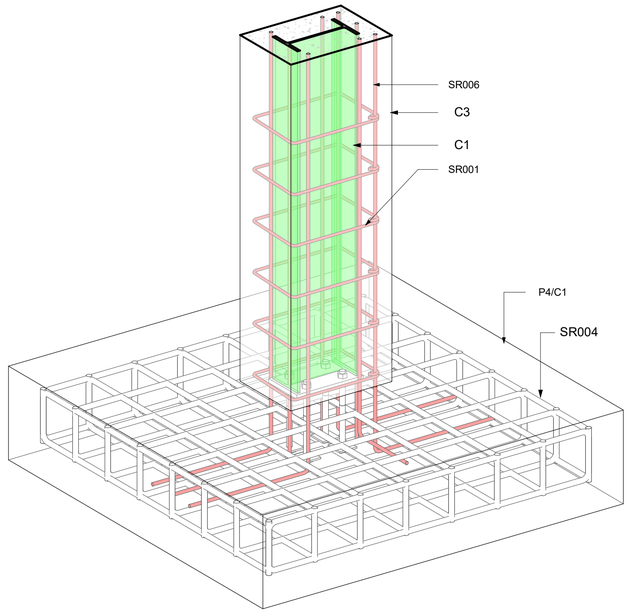
REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
P4/C1	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
SR001	HD10 TIES @ 200CRS
SR004	HD20 TOP AND BOTTOM BARS BOTHWAYS @ 150CRS
SR006	HD12 VERTICAL BARS, COLUMN ENCASEMENT



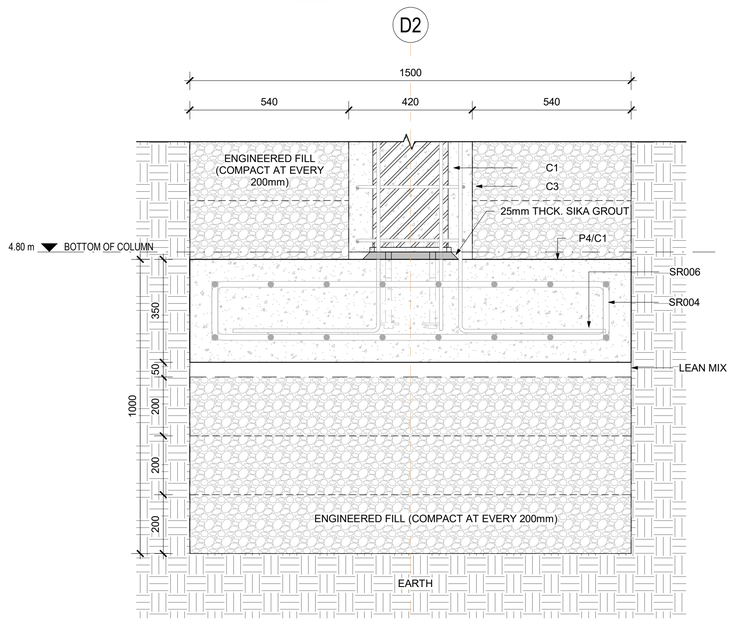
1 S33 - P4/C1 PLAN

500-03 Scale : 1 : 10



3 S33 - P4/C1 ISOMETRIC VIEW

Scale :



2 S33 - TYPICAL P4/C1 DETAIL

S33 Scale : 1 : 10

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

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Approved: NK	Date: ----	Plot Date:	
		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Job No: 22507209

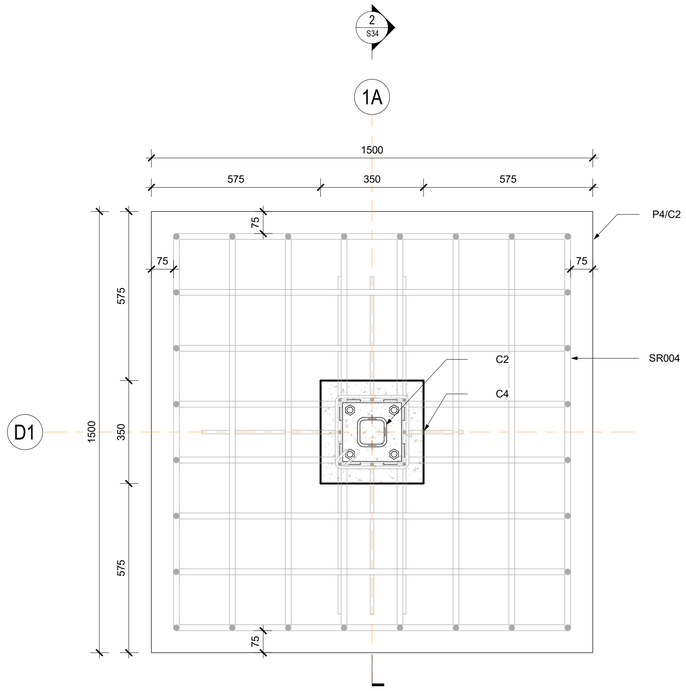
Drawing No: S33

Drawing Title:
TYPICAL P4/C1 FOOTING DETAILS

Scales:
A1 AS SHOWN
A3 HALF SHOWN

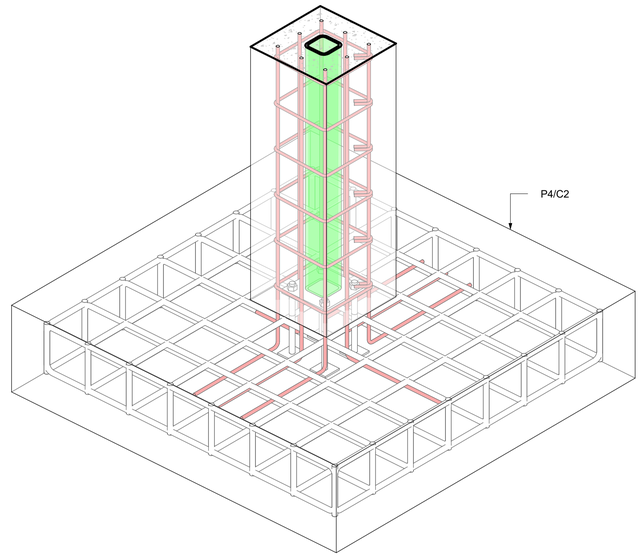
REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
P4/C2	1200 x 1200 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
SR004	HD20 TOP AND BOTTOM BARS BOTHWAYS @ 150CRS



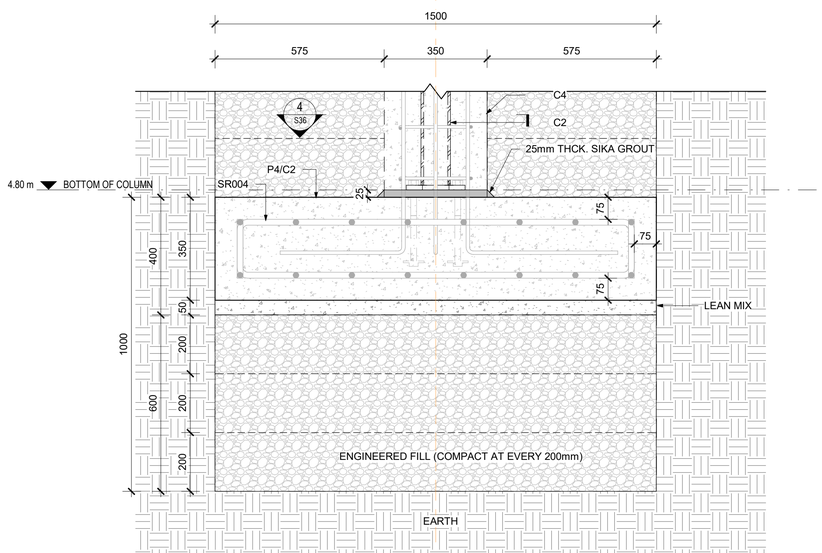
1 S34 - P4/C2 PLAN

500-03 Scale : 1 : 10



3 S34 - P4/C2 ISOMETRIC VIEW

Scale :



2 S34 - TYPICAL P4/C2 DETAIL

S34 Scale : 1 : 10

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

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Approved: NK	Date: ----	Plot Date:	
		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Job No: 22507209

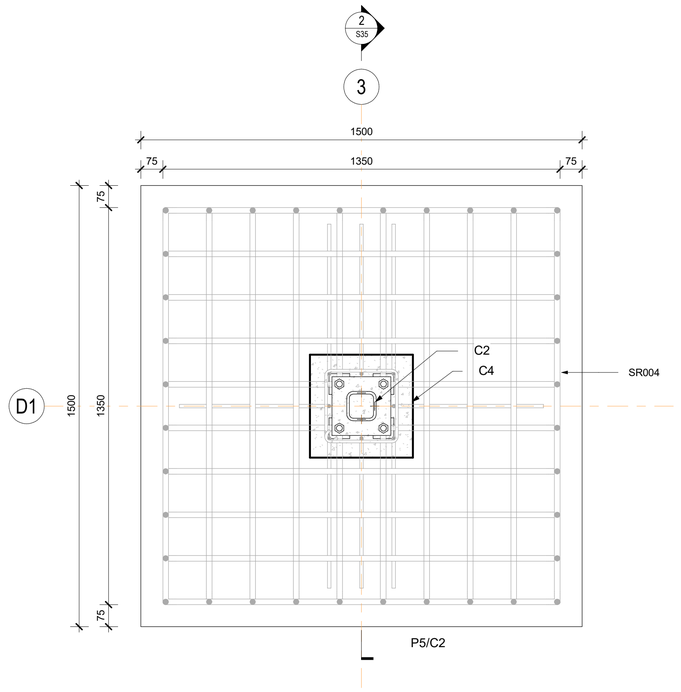
Drawing No: S34

Drawing Title:
TYPICAL P4/C2 FOOTING DETAILS

Scales:
A1 AS SHOWN
A3 HALF SHOWN

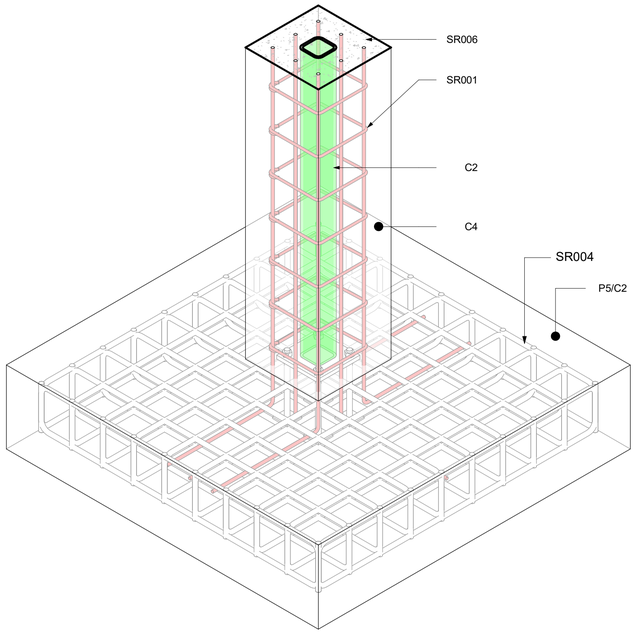
REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
P5/C2	1500 x 1500 x 350mm DEEP R.C PAD FOOTING REINFORCED WITH TOP AND BOTTOM HD20 @ 150mm CRS BOTHWAYS, 75mm COVER ALL AROUND
SR001	HD10 TIES @ 200CRS
SR004	HD20 TOP AND BOTTOM BARS BOTHWAYS @ 150CRS
SR006	HD12 VERTICAL BARS, COLUMN ENCASEMENT



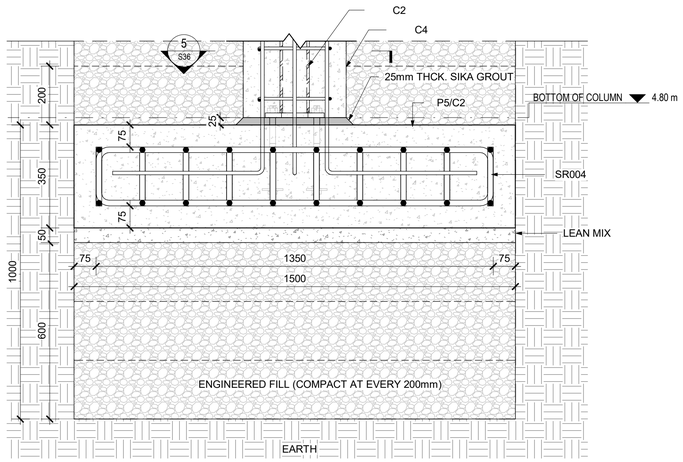
1 S35 - P5/C2 PLAN

500-03 Scale : 1 : 10



3 S35 - P5/C2 ISOMETRIC VIEW

Scale :



2 S35 - TYPICAL P5/C2 DETAIL

S35 Scale : 1 : 10

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

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		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

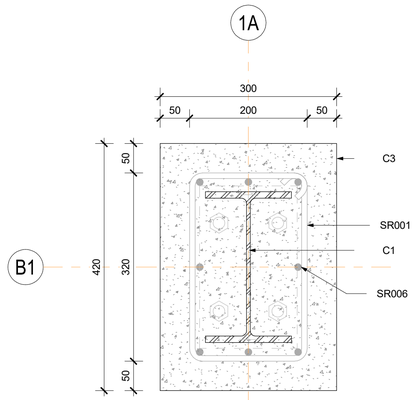
Drawing Title:
TYPICAL P5/C2 FOOTING DETAILS

Job No: 22507209

Drawing No: S35

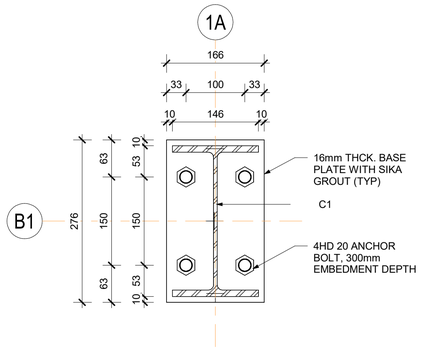
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REVISION A



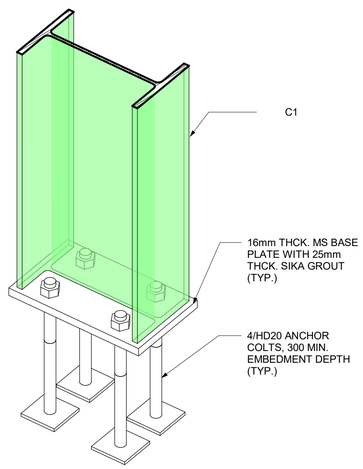
1 S36 - TYPICAL C1/C3 ENCASEMENT DETAIL

S30 Scale : 1 : 5



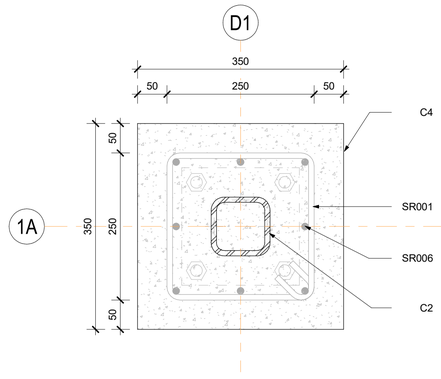
2 S36 - TYPICAL C1 BASE PLATE DETAIL

S30 Scale : 1 : 5



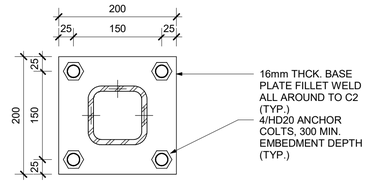
3 S36- C1 BASE PLATE VIEW

Scale :



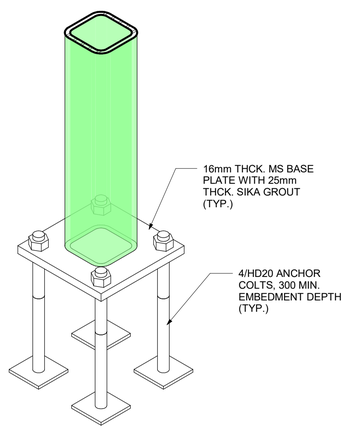
4 S36 - TYPICAL C2/C4 ENCASEMENT DETAIL

S34 Scale : 1 : 5



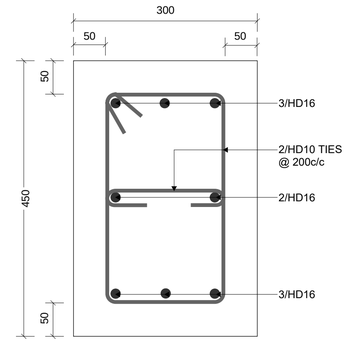
5 S36 - C2 - BASE PLATE DETAIL

S35 Scale : 1 : 5



6 S36 - C2 BASE PLATE VIEW

Scale :



7 TYPICAL TIE BEAM DETAIL

S36 SCALE 1: 5

SCHEDULES	
NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
C3	420 x 300 COLUMN ENCASEMENT
C4	350 x 350 COLUMN ENCASEMENT
SR001	HD10 TIES @ 200CRS
SR006	HD12 VERTICAL BARS, COLUMN ENCASEMENT

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Drawing Title:
TYPICAL ENCASEMENT DETAIL

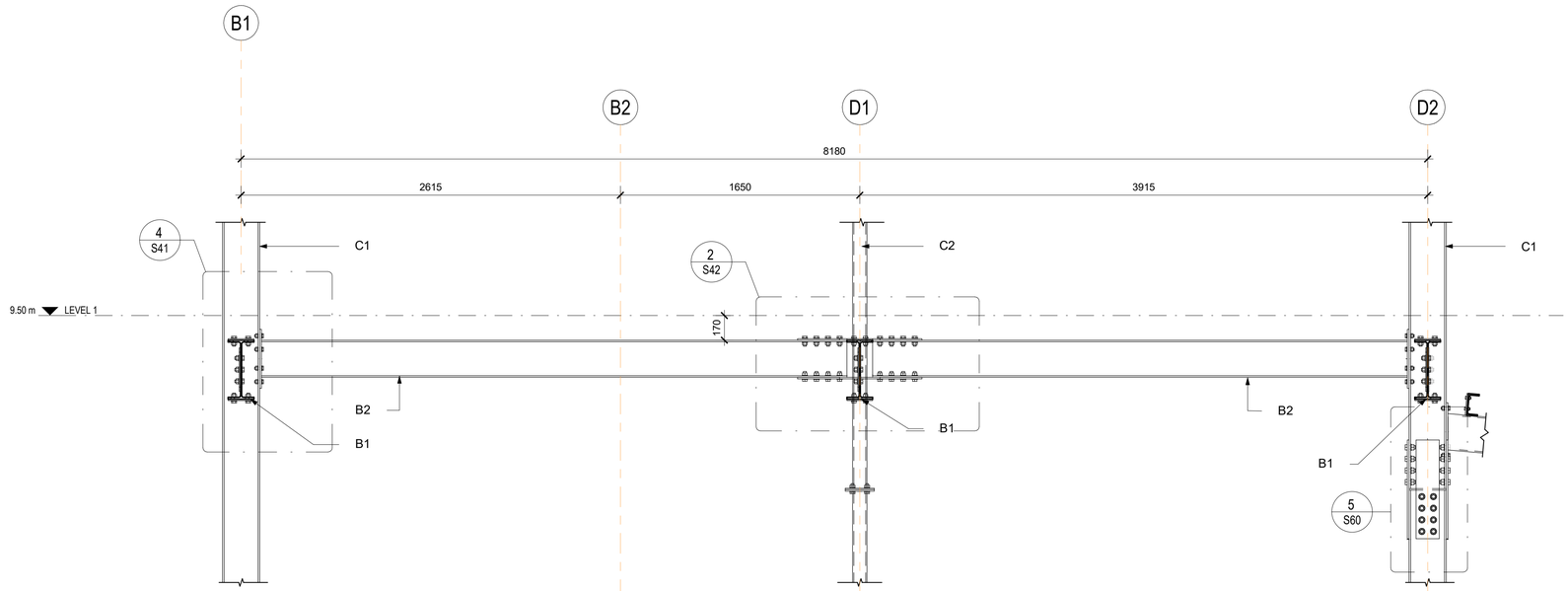
Job No: 22507209

Drawing No: S36

Scales:
A1 AS SHOWN
A3 HALF SHOWN

REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT



1 S40 - TYPICAL LEVEL 1 FRAMING SECTION

S05 Scale : 1 : 20

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REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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Designed:	RC	Date:	----
Approved:	NK	Date:	----

AUTOCAD REFERENCE
CAD File:
Plot By:
Plot Date:

QUALITY ASSURANCE
_ _ _ _ _ _ _ _ _ _ _ _
DIRECTOR DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

Job No: 22507209

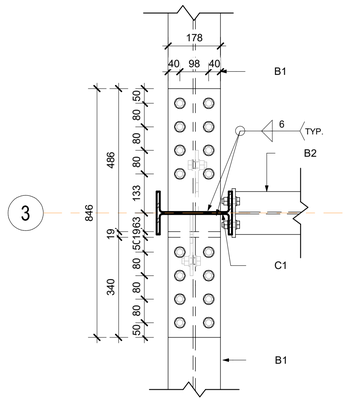
Drawing No: S40

Drawing Title:
TYPICAL LEVEL 1 FLOOR BEAM DETAILS

Scales:
A1 AS SHOWN
A3 HALF SHOWN

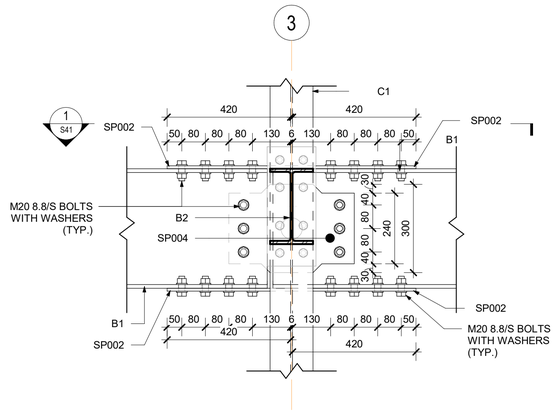
REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
SP002	10mm THCK. MS PLATE WITH 16-HD20 8.8/S BOLTS WITH WASHERS
SP004	10mm THCK. MS PLATE WITH 6-HD20 8.8/S BOLTS WITH WASHERS



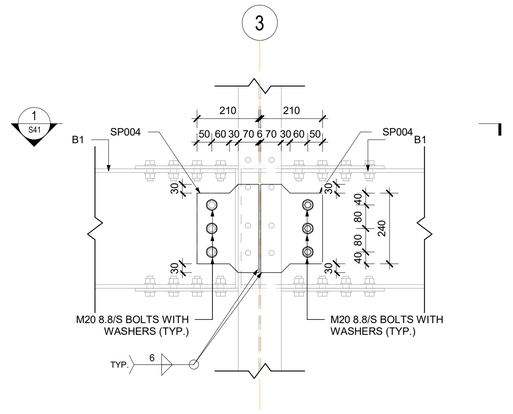
1 S41 - B1/B2 TO C1 PLAN DETAIL

S41 Scale : 1 : 10



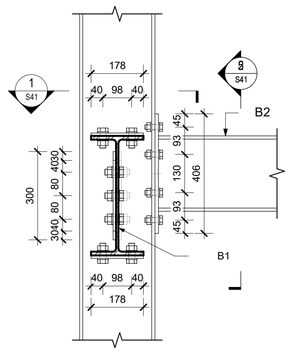
2 S41 - TYPICAL B1 TO C1 DETAIL

S41 Scale : 1 : 10



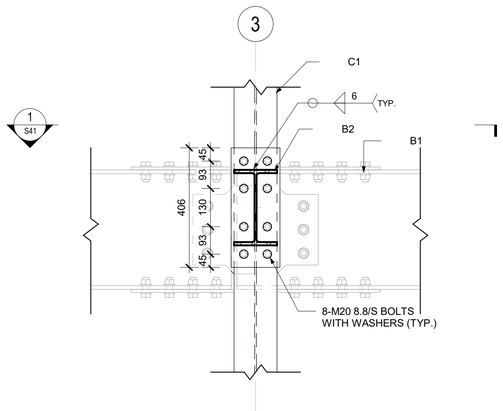
3 S41 - SP004 PLATE DETAIL

Scale : 1 : 10



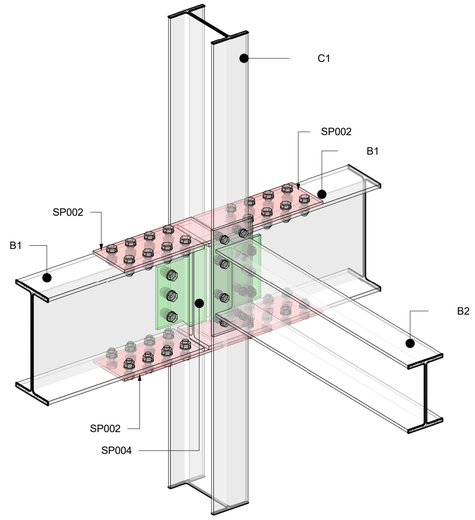
4 S40 - TYPICAL B1/B2 TO C1 DETAIL

S40 Scale : 1 : 10



5 S41 - TYPICAL B2 TO C1 DETAL

S41 Scale : 1 : 10



6 S41 - TYPICAL B1/B2 TO C1 VIEW

Scale :

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE	
Checked: RC	Date: 08/14/25	CAD File:	
Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	

QUALITY ASSURANCE

DIRECTOR DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Drawing Title:
TYPICAL B1/B2 TO C1 DETAILS

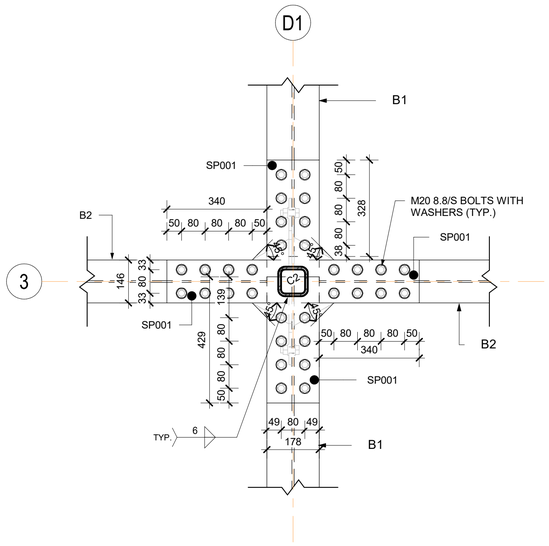
Job No: 22507209

Drawing No: S41

Scales: A1 AS SHOWN, A3 HALF SHOWN

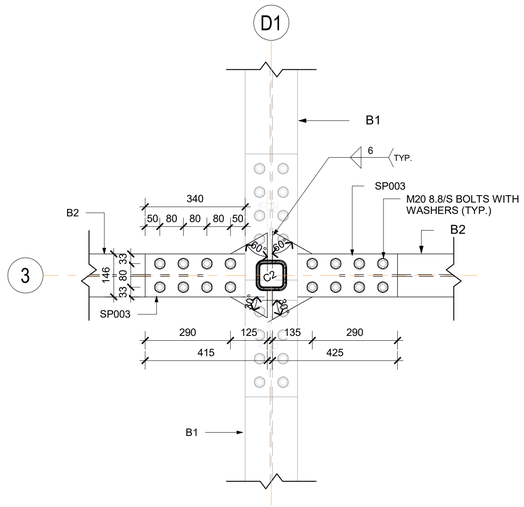
REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
SP001	10mm THCK. MS PLATE WITH 24-HD20 8.8/S BOLTLS WITH WASHERS
SP002	10mm THCK. MS PLATE WITH 16-HD20 8.8/S BOLTS WITH WASHERS
SP003	10mm THCK. MS PLATE WITH 16-HD20 8.8/S BOLTS WITH WASHERS
SP004	10mm THCK. MS PLATE WITH 6-HD20 8.8/S BOLTS WITH WASHERS



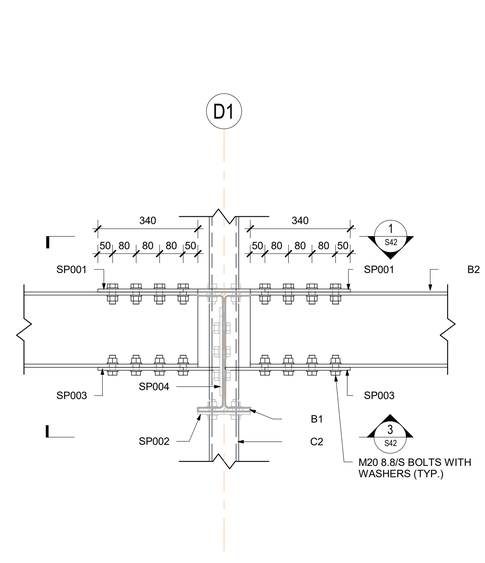
1 S42 - B1/B2 TO C2 PLAN DETAIL

S42 Scale : 1 : 10



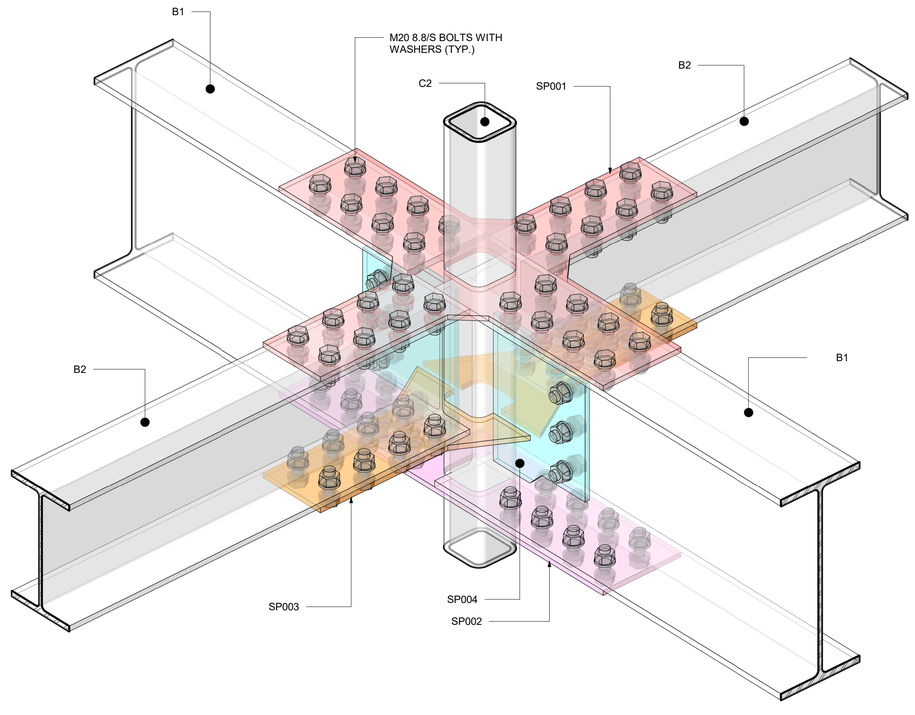
3 S42 - SP003 PLATE DETAIL

S42 Scale : 1 : 10



2 S42 - TYPICAL B2 TO C2 DETAIL

S40 Scale : 1 : 10



4 S42 - TYPICAL B1/B2 TO C2 VIEW

Scale :

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REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
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PHONE: (679) 3313 388, FAX (679) 3302903
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Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	
		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

Job No: 22507209

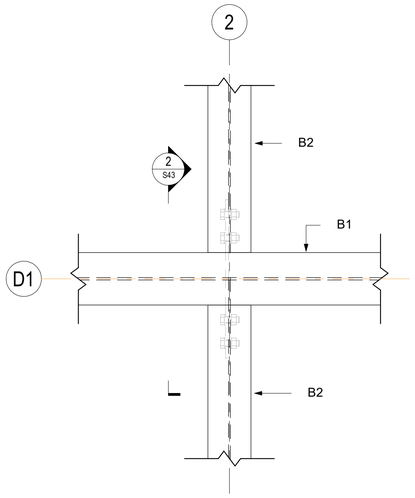
Drawing No: S42

Drawing Title:
TYPICAL B1/B2 TO C2 DETAILS

Scales: A1 AS SHOWN; A3 HALF SHOWN

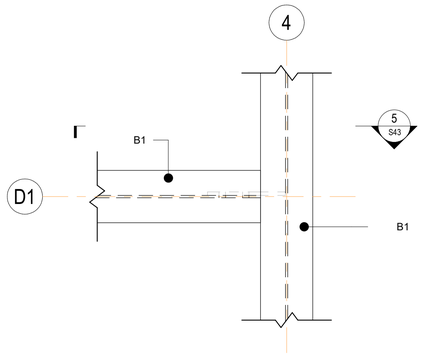
REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM



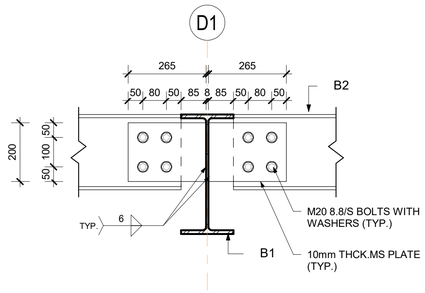
1 S43 - TYPICAL B2 TO B3 PLAN

Scale : 1 : 10



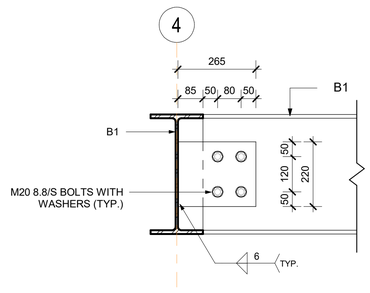
4 S43 - TYPICAL B1 TO B1 PLAN

Scale : 1 : 10



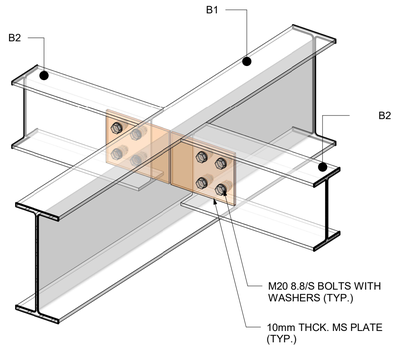
2 S43 - TYPICAL B2 TO B1 DETAIL

S43 Scale : 1 : 10



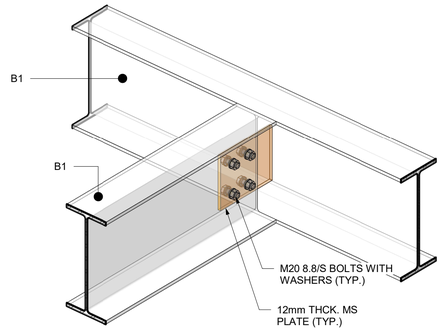
5 S43 - TYPICAL B1 TO B1 DETAIL

S43 Scale : 1 : 10



3 S43 - TYPICAL B2 TO B1 VIEW

Scale :



6 S43 - TYPICAL B1 TO B1 VIEW

Scale :

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	A	EA	24.11.2025	FOR TENDER	RC	NK

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CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
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P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

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Approved: NK	Date: ----	Plot Date:	

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_____ DIRECTOR _____ DATE

Project Name: CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI RD SOLOMON ISLAND

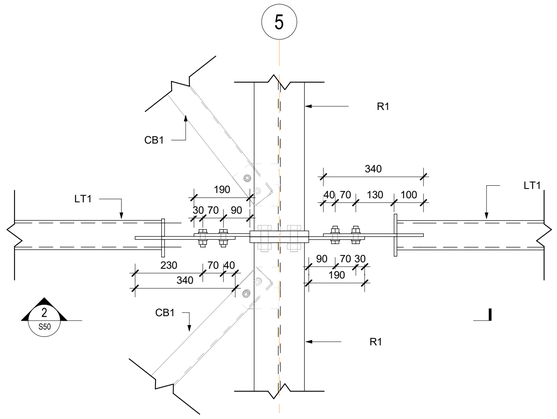
Job No: 22507209

Drawing No: S43

Drawing Title: TYPICAL FLOOR FRAMING DETAILS

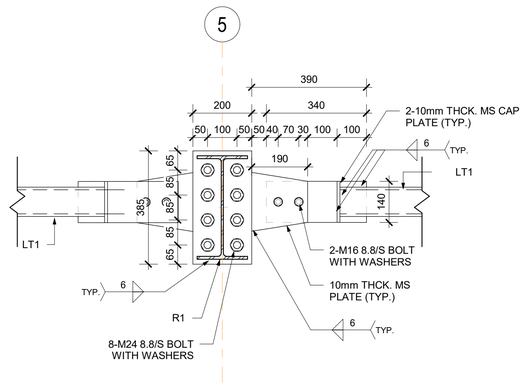
Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A



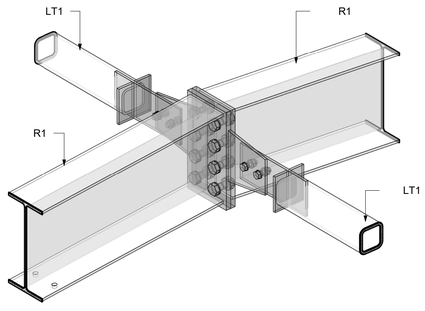
1 S50 - TYPICAL APEX PLAN

Scale : 1 : 10



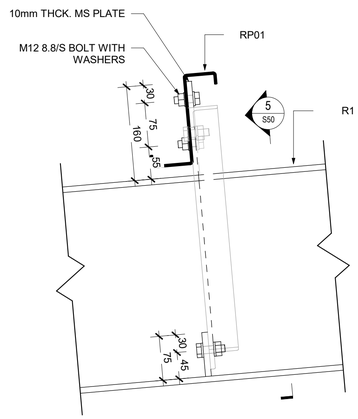
2 S50- TYPICAL APEX DETAIL

S50 Scale : 1 : 10



3 S50 - TYPICAL APEX VIEW

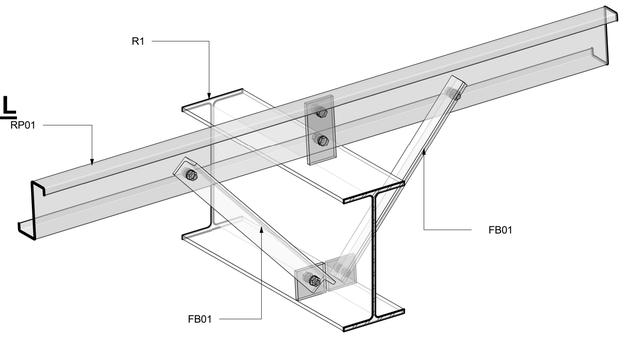
Scale :



4 S50 - TYPICAL PURLIN AND FLYBRACE DETAIL

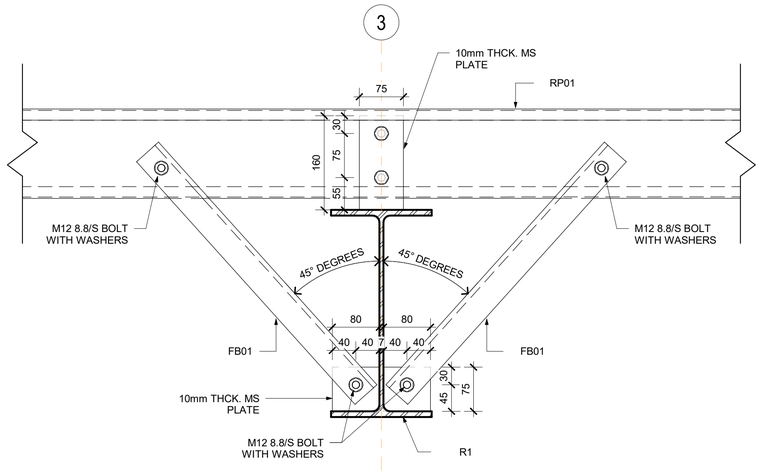
Scale : 1 : 5

SCHEDULES	
NUMBER	DESCRIPTION
CB1	75EA 9.0 STEEL ROOF BRACING
FB01	50EA 9.0 STEEL ROOF FLY BRACING
LT1	100SHS 9.0 STEEL ROOF LATERAL TIE
R1	360UB 37.3 STEEL RAFTER
RP01	Z15075 STEEL PURLINS @ 600 - 900CRS FIXED ONTO STEEL RAFTER USING 8mm THICK MS PLATE WELDED ONTO STEEL RAFTERS AND FIXED TO THE PURLINS WITH 2-4 No. M12 8.8/S BOLTS PER CONNECTION



6 S50 - TYPICAL PURLIN AND FLYBRACE VIEW

Scale :



5 S50 - TYPICAL PURLIN TO RAFTER CONNECTION

S50 Scale : 1 : 5

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REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

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		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

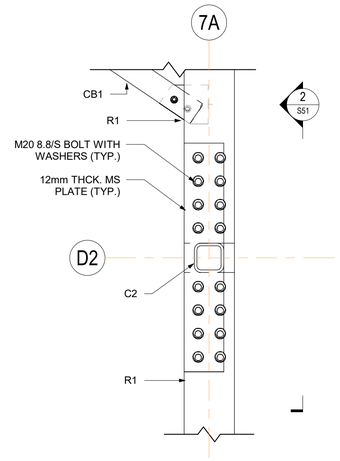
Drawing Title:
TYPICAL ROOF DETAILS (SHEET 1 OF 2)

Job No: 22507209

Drawing No: S50

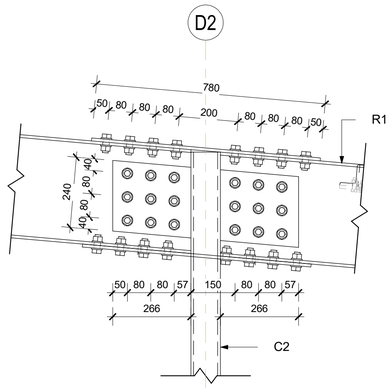
Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A



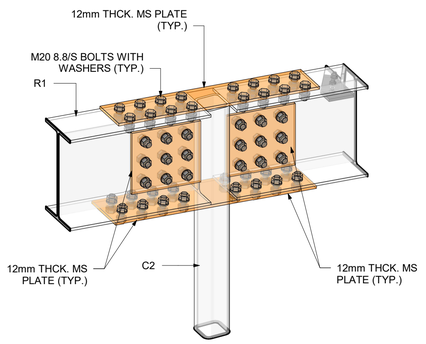
1 S51 - TYPICAL R1 TO C2 PLAN

S51 Scale : 1 : 10



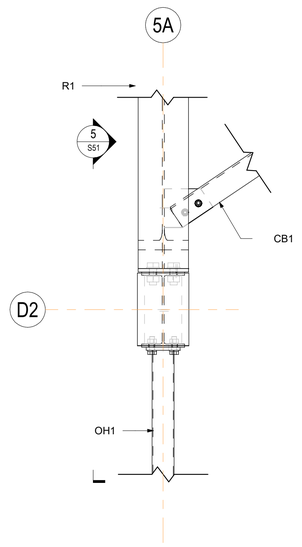
2 S51- TYPICAL R1 TO C2 DETAIL

S51 Scale : 1 : 10



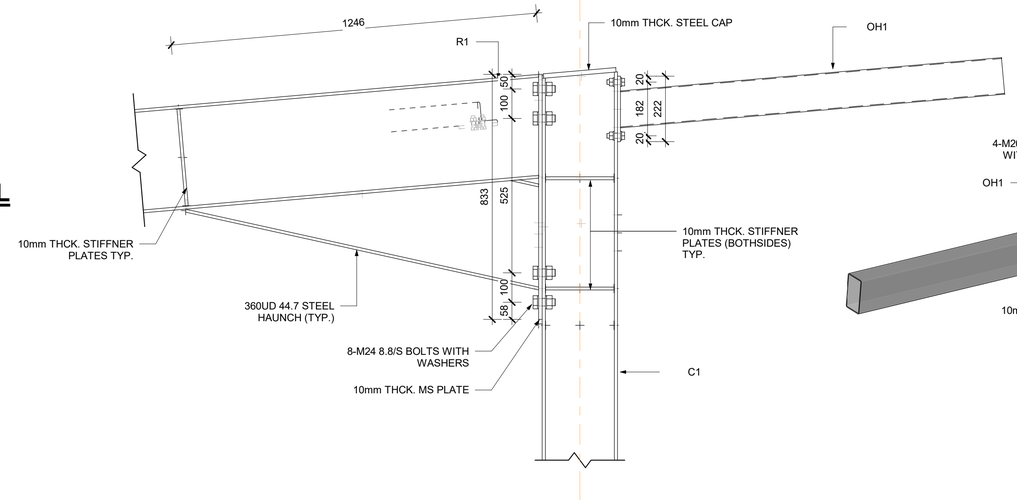
3 S51 - TYPICAL R1 TO C2 VIEW

Scale :



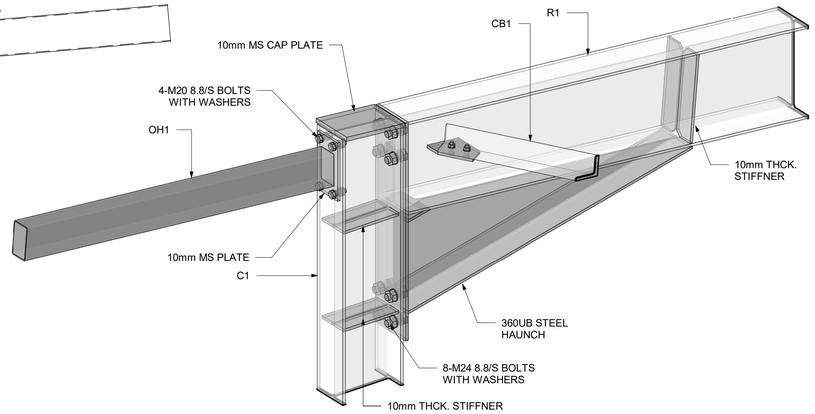
4 S51 - TYPICAL KNEE DETAIL PLAN

S51 Scale : 1 : 10



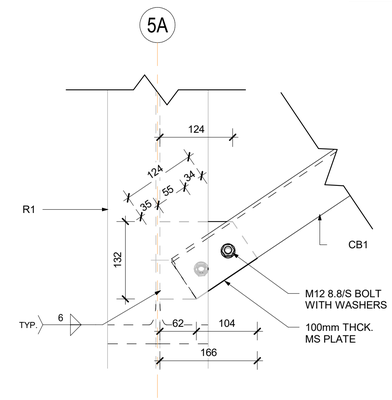
5 S51 - TYPICAL R1 TO C1 KNEE DETAIL

S51 Scale : 1 : 10



6 S51 - TYPICAL R1 TO C1 KNEE VIEW

Scale :



7 S51 - TYPICAL ROOF CROSS BRACING PLAN

Scale : 1 : 5

SCHEDULES	
NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
CB1	75EA 9.0 STEEL ROOF BRACING
OH1	150PFC STEEL OUTRIGGER RAFTER 1300mm OVERHANG
R1	360UB 37.3 STEEL RAFTER

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REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
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EMAIL: info@nrwmacallan.com.fj

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		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

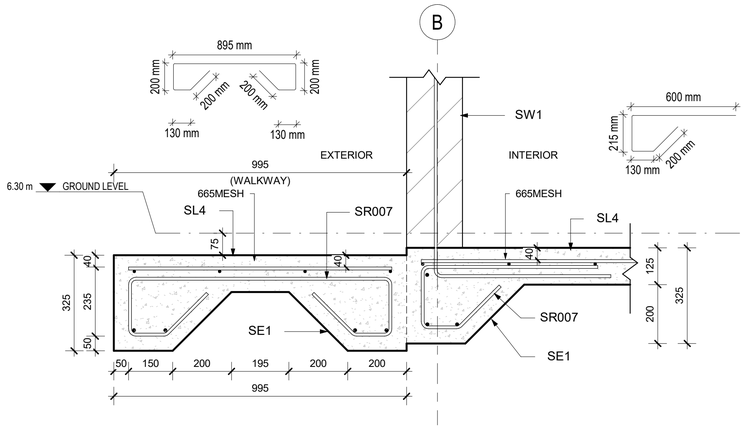
Drawing Title:
TYPICAL ROOF DETAILS (SHEET 2 OF 2)

Job No: 22507209

Drawing No: S51

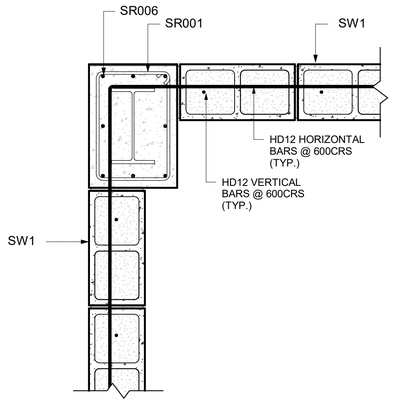
Scales: A1 AS SHOWN; A3 HALF SHOWN

REVISION A



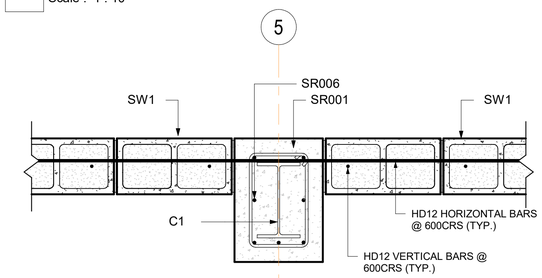
1 S60 - TYPICAL SLAB EDGE DETAIL

Scale : 1 : 10



2 S60 - TYPICAL WALL TO COLUMN CORNER DETAIL

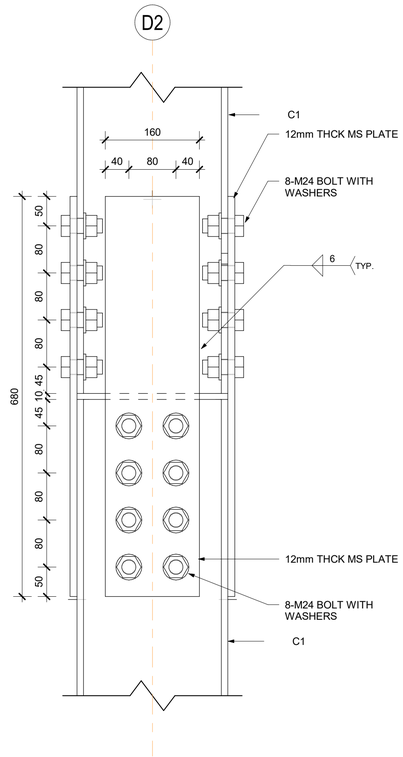
Scale : 1 : 10



4 S60 - TYPICAL WALL TO COLUMN JUNCTION DETAIL

Scale : 1 : 10

SCHEDULES	
NUMBER	DESCRIPTION
C1	250UB 37.3 STEEL COLUMN WITH ENCASEMENT
SE1	R.C SLAB EDGE
SL4	125mm THCK. R.C SLAB WITH 665MESH 40CVR
SR001	HD10 TIES @ 200CRS
SR006	HD12 VERTICAL BARS, COLUMN ENCASEMENT
SR007	HD10 @ 200CRS
SW1	150mm THK MASONRY WALL (FULLY GROUTED) WITH A 15mm THCK. MIN. PLASTER



5 S60 - TYPICAL C1 SPLICE DETAIL

S40 Scale : 1 : 5

3 S50 - TYPICAL DISH FOOTING DETAIL

Scale : 1 : 10

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CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Drawing Title:
TYPICAL DETAILS

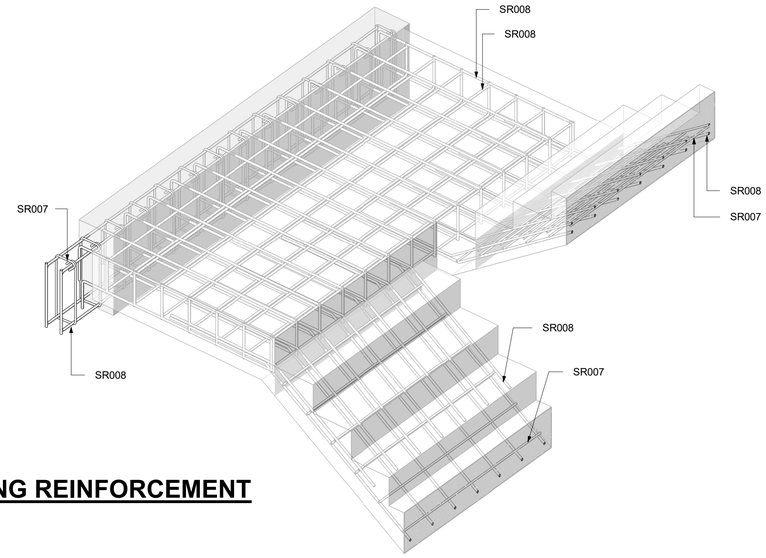
Job No: 22507209

Drawing No: S60

Scales: A1 AS SHOWN, A3 HALF SHOWN

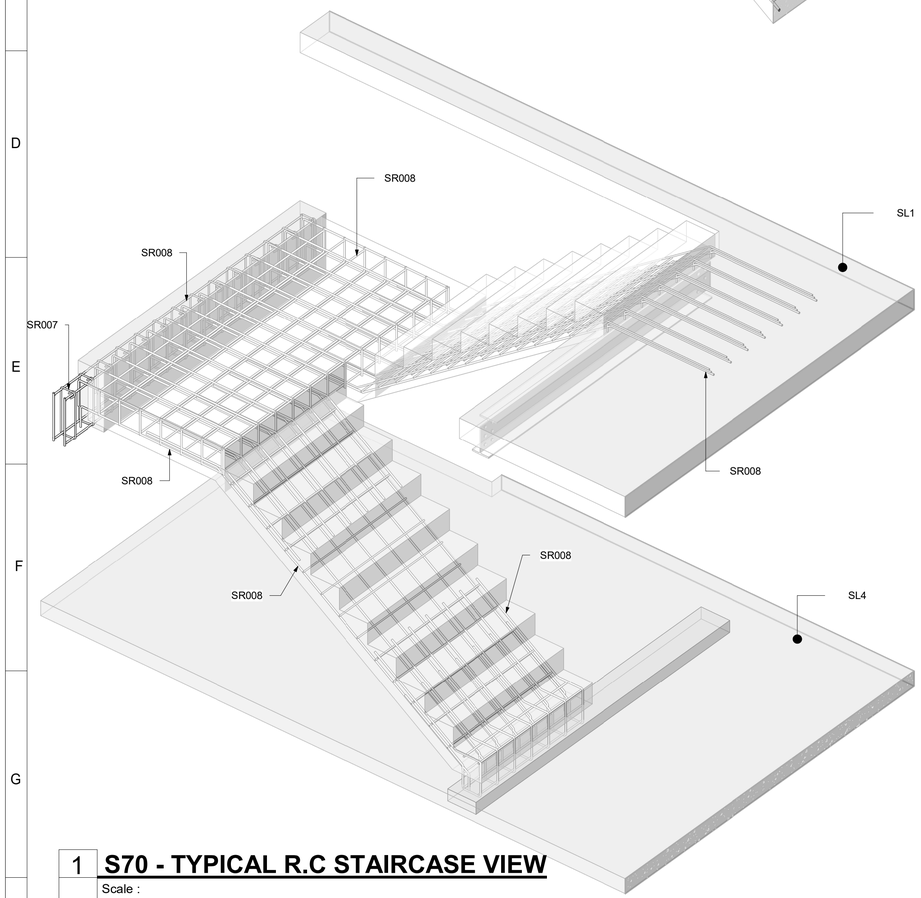
REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B2	250UB 37.3 STEEL FLOOR BEAM
SL1	170mm THCK. R.C BONDEK SLAB
SL4	125mm THCK. R.C SLAB WITH 665MESH 40CVR
SR007	HD10 @ 200CRS
SR008	HD12 @ 200CRS
SW1	150mm THK MASONRY WALL (FULLY GROUTED) WITH A 15mm THCK. MIN. PLASTER



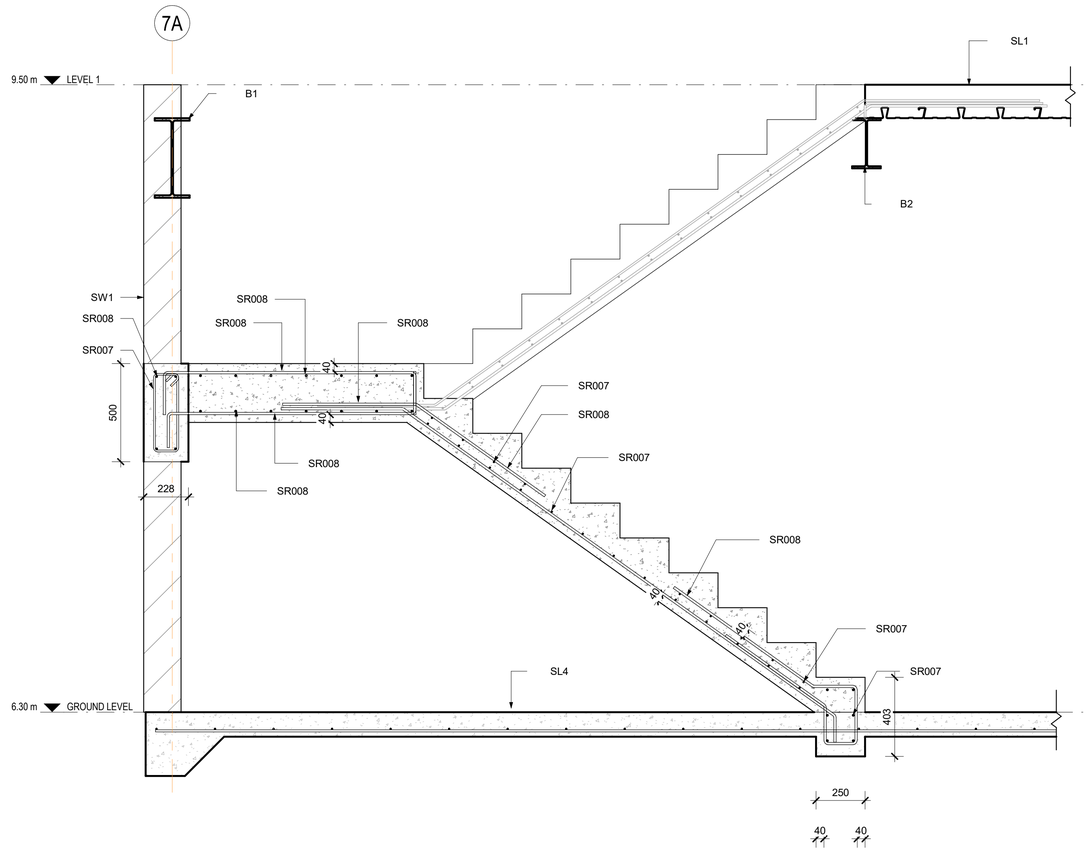
2 S70 - TYPICAL LANDING REINFORCEMENT

Scale :



1 S70 - TYPICAL R.C STAIRCASE VIEW

Scale :



3 S70 - R.C STAIRCASE SECTION

Scale : 1 : 15

FOR TENDER

REVISIONS	No	By	Date	Description	Ckd	Appr
	A	EA	24.11.2025	FOR TENDER	RC	NK

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Client

fmc PACIFIC

NRW MACALLAN (FIJI) LTD
CONSULTING ENGINEERS
CIVIL,STRUCTURAL,ELECTRICAL,MECHANICAL & PROJECT MANAGEMENT
79 RATU MARA ROAD, SAMABULA, SUVA
P.O. BOX 1208, SUVA
PHONE: (679) 3313 388, FAX (679) 3302903
EMAIL: info@nrwmacallan.com.fj

Drafter: EA	Date: 08/14/25	AUTOCAD REFERENCE	
Checked: RC	Date: 08/14/25	CAD File:	
Designed: RC	Date: ----	Plot By:	
Approved: NK	Date: ----	Plot Date:	
		QUALITY ASSURANCE	
		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Drawing Title:
TYPICAL R.C STAIRCASE DETAILS

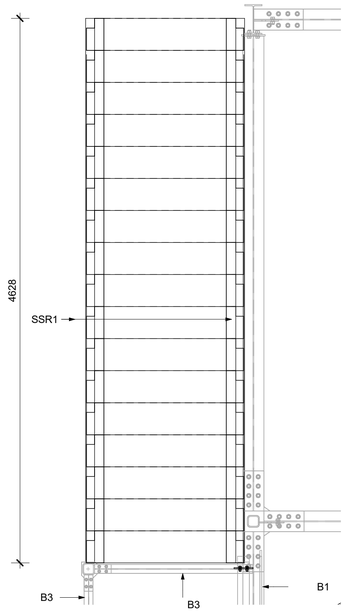
Job No: 22507209

Drawing No: S70

Scales: A1 AS SHOWN; A3 HALF SHOWN

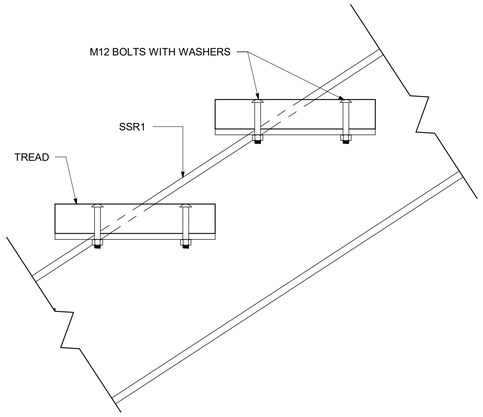
REVISION A

SCHEDULES	
NUMBER	DESCRIPTION
B1	410UB 59.7 STEEL FLOOR BEAM
B3	150UB 15.0 STEEL FLOOR BEAM
C2	100SHS 9.0 STEEL COLUMN WITH ENCASEMENT
SL1	170mm THCK. R.C BONDEK SLAB
SSR1	250PFC STEEL STRINGER, 6mM FILLET WELD TO STEEL COLUMN



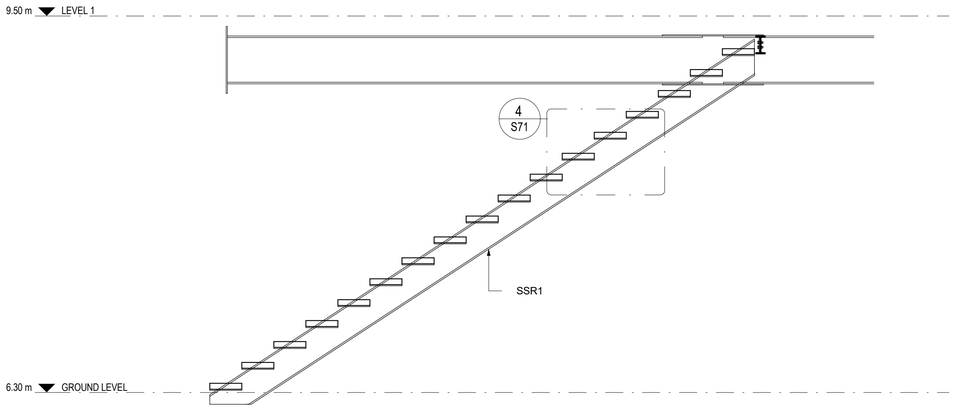
3 S71 - EMERGENCY STAIRCASE PLAN

Scale : 1 : 25



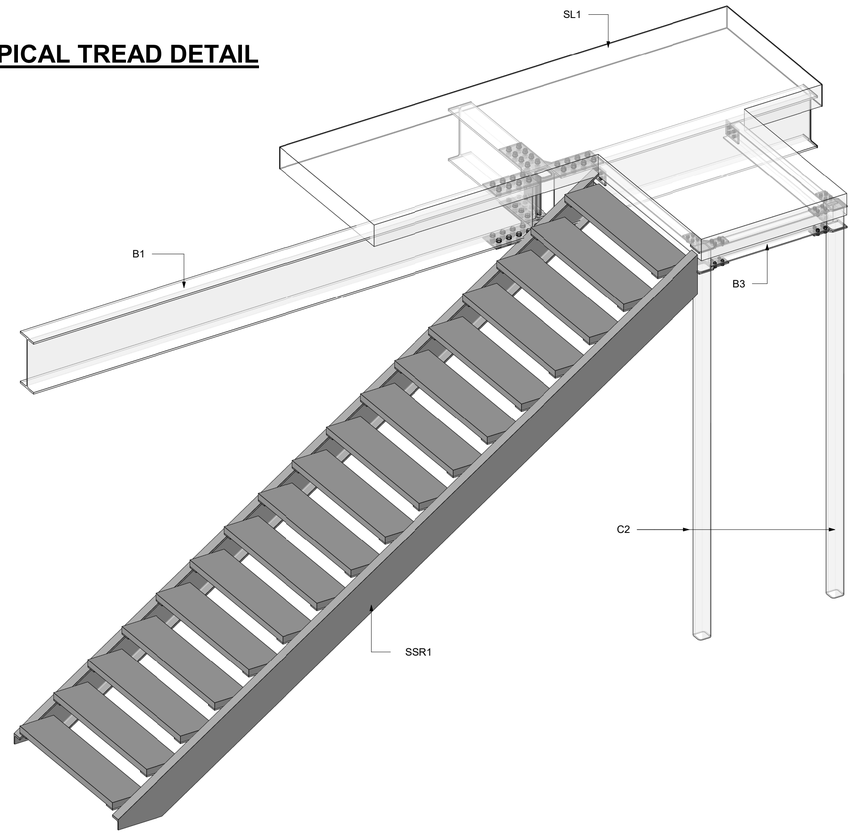
4 S71 - TYPICAL TREAD DETAIL

S71 Scale : 1 : 5



2 S71 - EMERGENCY STAIRCASE SECTION

Scale : 1 : 25



1 S71 - TYPICAL EMERGENCY STAIRCASE VIEW

Scale :

FOR TENDER

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		DIRECTOR	DATE

Project Name:
CIVIL & STRUCTURAL DESIGN OF MFMR PROVINCIAL OFFICE AUKI-HAUHUI
RD SOLOMON ISLAND

Job No: 22507209

Drawing No: S71

Drawing Title:
TYPICAL EMERGENCY STAIRCASE DETAILS

Scales:
A1 AS SHOWN
A3 HALF SHOWN

REVISION A